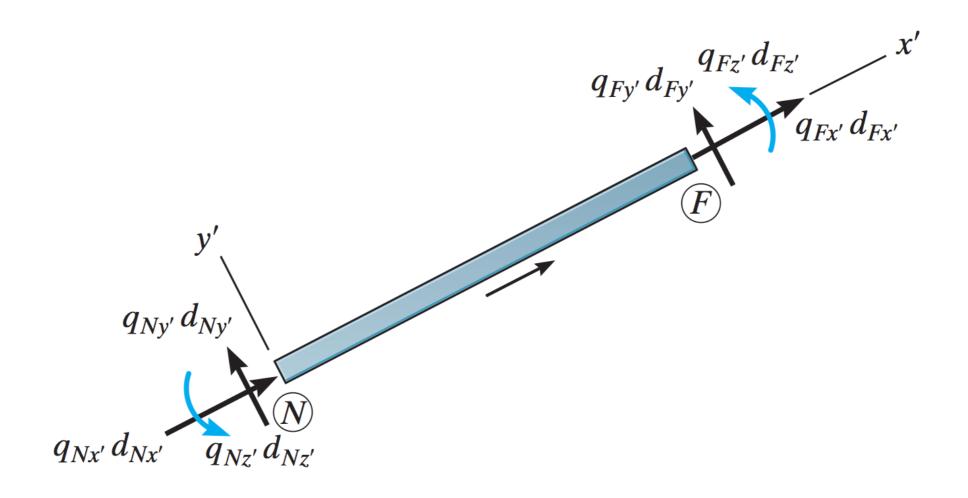
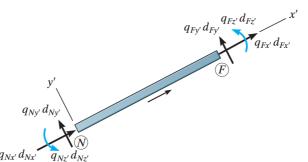
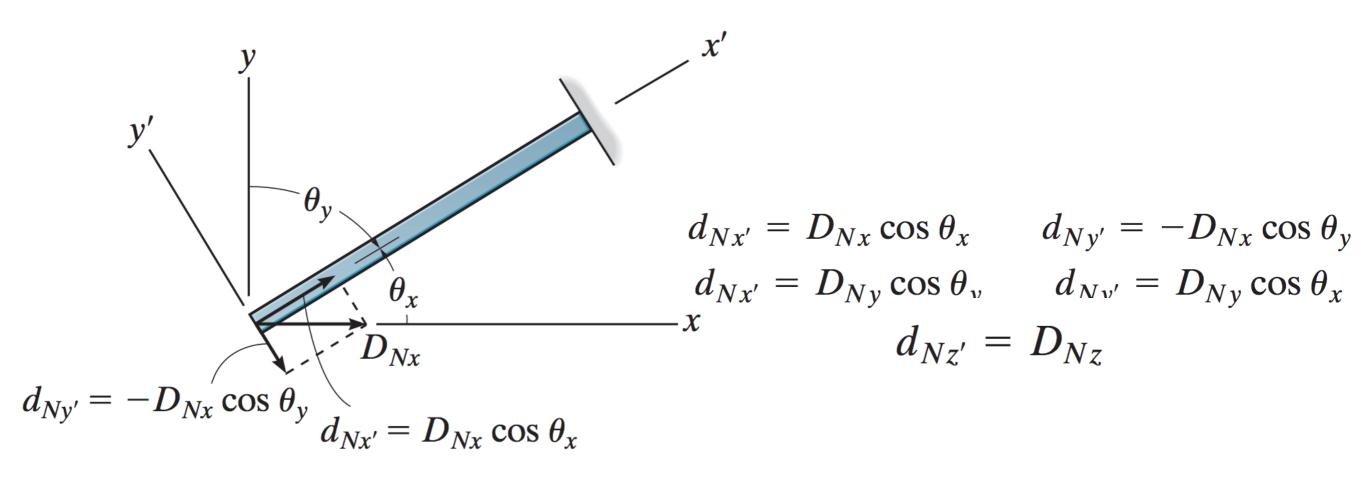
# Direct Stiffness for Frames



# q = k'd



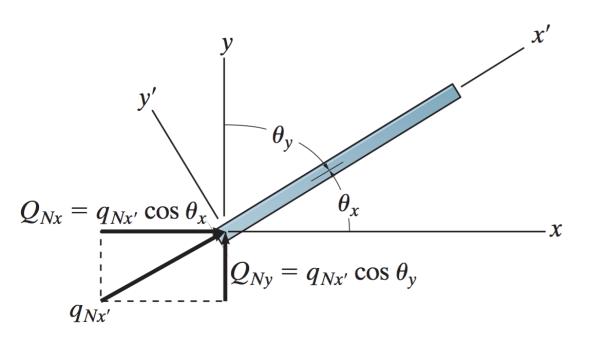
$q_{Nx'}d_{Nx'} = q_{Nz'}d_{Nz'}$											
	$\lceil q_{Nx'}  ceil$	$\frac{AE}{L}$	0	0	$-\frac{AE}{L}$	0	0	$\left\lceil d_{Nx'} ight ceil$			
	$\left q_{Ny'} ight $	0	$\frac{12EI}{L^3}$	$\frac{6EI}{L^2}$	0	$-\frac{12EI}{L^3}$	$\frac{6EI}{L^2}$	$\left  d_{Ny'}  ight $			
	$\left q_{Nz'} ight $	0	$\frac{6EI}{L^2}$	$\frac{4EI}{L}$	0	$-\frac{6EI}{L^2}$	$\frac{2EI}{L}$	$\left  d_{Nz'}  ight $			
	$\left  \begin{array}{c} q_{Fx'} \end{array} \right  =$	$-rac{AE}{L}$	0	0	$\frac{AE}{L}$	0	0	$\left  d_{Fx'}  ight $			
	$\left  q_{Fy'}  ight $	0	$-\frac{12EI}{L^3}$	$-\frac{6EI}{L^2}$	0	$\frac{12EI}{L^3}$	$-\frac{6EI}{L^2}$	$\left  d_{Fy'}  ight $			
	$oxed{q_{Fz'}}$	0	$\frac{6EI}{L^2}$	$\frac{2EI}{L}$	0	$-\frac{6EI}{L^2}$	$\frac{4EI}{L}$	$bgg[d_{Fz'}bgg]$			



$$y'$$
 $d_{Fx'} = D_{Fx} \cos \theta_x$ 
 $d_{Fy'} = -D_{Fx} \cos \theta_y$ 
 $d_{Fx'} = D_{Fy} \cos \theta_y$ 
 $d_{Fx'} = D_{Fy} \cos \theta_y$ 
 $d_{Fx'} = D_{Fy} \cos \theta_y$ 
 $d_{Fx'} = D_{Fx} \cos \theta_y$ 
 $d_{Fx'} = D_{Fx} \cos \theta_y$ 
 $d_{Fx'} = D_{Fx} \cos \theta_y$ 

$$\begin{bmatrix} d_{Nx'} \\ d_{Ny'} \\ d_{Nz'} \\ d_{Fx'} \\ d_{Fz'} \end{bmatrix} = \begin{bmatrix} \lambda_x & \lambda_y & 0 & 0 & 0 & 0 \\ -\lambda_y & \lambda_x & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & \lambda_x & \lambda_y & 0 \\ 0 & 0 & 0 & -\lambda_y & \lambda_x & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix} \begin{bmatrix} D_{Nx} \\ D_{Ny} \\ D_{Nz} \\ D_{Fx} \\ D_{Fy} \\ D_{Fz} \end{bmatrix}$$

 $\mathbf{d} = \mathbf{T}\mathbf{D}$ 



$$Q_{Nx} = q_{Nx'} \cos \theta_x$$
  $Q_{Ny} = q_{Nx'} \cos \theta_y$   $Q_{Nx} = -q_{Ny'} \cos \theta_y$   $Q_{Ny} = q_{Ny'} \cos \theta_x$   $Q_{Nz} = q_{Nz'}$ 

$$Q_{Nx} = -q_{Ny'} \cos \theta_y Q_{Ny} = q_{Ny'} \cos \theta_x$$

$$Q_{Fx} = q_{Fx'} \cos \theta_x$$
  $Q_{Fy} = q_{Fx'} \cos \theta_y$   $Q_{Fx} = -q_{Fy'} \cos \theta_y$   $Q_{Fy} = q_{Fy'} \cos \theta_x$   $Q_{Fz} = q_{Fz'}$ 

$$\begin{bmatrix} Q_{Nx} \\ Q_{Ny} \\ Q_{Nz} \\ Q_{Fx} \\ Q_{Fy} \\ Q_{Fz} \end{bmatrix} = \begin{bmatrix} \lambda_x & -\lambda_y & 0 & 0 & 0 & 0 \\ \lambda_y & \lambda_x & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & \lambda_x & -\lambda_y & 0 \\ 0 & 0 & 0 & \lambda_y & \lambda_x & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix} \begin{bmatrix} q_{Nx'} \\ q_{Ny'} \\ q_{Nz'} \\ q_{Fx'} \\ q_{Fy'} \\ q_{Fz'} \end{bmatrix}$$

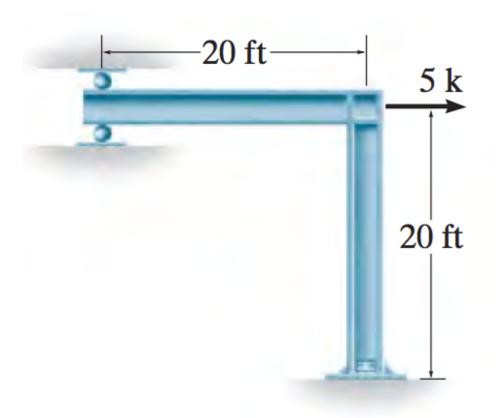
$$\mathbf{Q} = \mathbf{T}^T \mathbf{q}$$

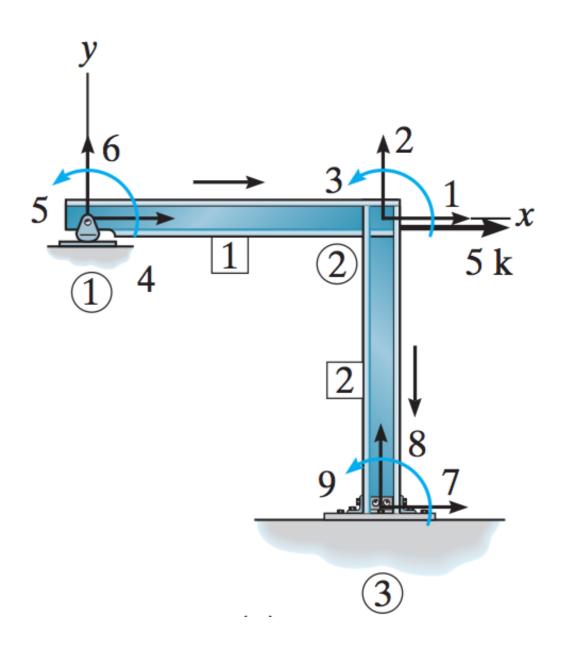
## $\mathbf{k} = \mathbf{T}^T \mathbf{k}' \mathbf{T}$

## () = k

$$\mathbf{k} = \begin{bmatrix} N_{x} & N_{y} & N_{z} & F_{x} & F_{y} & F_{z} \\ \frac{AE}{L}\lambda_{x}^{2} + \frac{12EI}{L^{3}}\lambda_{y}^{2} & \left(\frac{AE}{L} - \frac{12EI}{L^{3}}\right)\lambda_{x}\lambda_{y} & -\frac{6EI}{L^{2}}\lambda_{y} & -\left(\frac{AE}{L}\lambda_{x}^{2} + \frac{12EI}{L^{3}}\lambda_{y}^{2}\right) & -\left(\frac{AE}{L} - \frac{12EI}{L^{3}}\right)\lambda_{x}\lambda_{y} & -\frac{6EI}{L^{2}}\lambda_{y} \\ \frac{AE}{L} - \frac{12EI}{L^{3}}\lambda_{x}\lambda_{y} & \left(\frac{AE}{L}\lambda_{y}^{2} + \frac{12EI}{L^{3}}\lambda_{x}^{2}\right) & \frac{6EI}{L^{2}}\lambda_{x} & -\left(\frac{AE}{L} - \frac{12EI}{L^{3}}\right)\lambda_{x}\lambda_{y} & -\left(\frac{AE}{L}\lambda_{y}^{2} + \frac{12EI}{L^{3}}\lambda_{x}^{2}\right) & \frac{6EI}{L^{2}}\lambda_{x} \\ - \frac{6EI}{L^{2}}\lambda_{y} & \frac{6EI}{L^{2}}\lambda_{x} & \frac{4EI}{L} & \frac{6EI}{L^{2}}\lambda_{y} & -\frac{6EI}{L^{2}}\lambda_{x} & \frac{2EI}{L} \\ -\left(\frac{AE}{L}\lambda_{x}^{2} + \frac{12EI}{L^{3}}\lambda_{y}^{2}\right) & -\left(\frac{AE}{L}\lambda_{y}^{2} + \frac{12EI}{L^{3}}\lambda_{x}^{2}\right) & -\frac{6EI}{L^{2}}\lambda_{x} & \left(\frac{AE}{L} - \frac{12EI}{L^{3}}\lambda_{x}\lambda_{y}\right) & \left(\frac{AE}{L}\lambda_{y}^{2} + \frac{12EI}{L^{3}}\lambda_{x}^{2}\right) & -\frac{6EI}{L^{2}}\lambda_{x} \\ -\left(\frac{AE}{L} - \frac{12EI}{L^{3}}\right)\lambda_{x}\lambda_{y} & -\left(\frac{AE}{L}\lambda_{y}^{2} + \frac{12EI}{L^{3}}\lambda_{x}^{2}\right) & -\frac{6EI}{L^{2}}\lambda_{x} & \left(\frac{AE}{L} - \frac{12EI}{L^{3}}\right)\lambda_{x}\lambda_{y} & \left(\frac{AE}{L}\lambda_{y}^{2} + \frac{12EI}{L^{3}}\lambda_{x}^{2}\right) & -\frac{6EI}{L^{2}}\lambda_{x} \\ -\frac{6EI}{L^{2}}\lambda_{y} & \frac{6EI}{L^{2}}\lambda_{x} & \frac{2EI}{L} & \frac{6EI}{L^{2}}\lambda_{y} & -\frac{6EI}{L^{2}}\lambda_{x} & \frac{4EI}{L} \end{bmatrix} F_{z}$$

Determine the loadings at the joints of the two-member frame shown in Fig. 16–4a. Take  $I = 500 \text{ in}^4$ ,  $A = 10 \text{ in}^2$ , and  $E = 29(10^3)$  ksi for both members.





$$\mathbf{D}_{k} = \begin{bmatrix} 0 \\ 0 \\ 0 \\ 0 \\ 0 \end{bmatrix} \begin{matrix} 6 \\ 7 \\ 8 \\ 9 \end{matrix} \qquad \mathbf{Q}_{k} = \begin{bmatrix} 5 & 1 \\ 0 & 2 \\ 0 & 3 \\ 0 & 4 \\ 5 \end{bmatrix}$$

$$\frac{AE}{L} = \frac{10[29(10^3)]}{20(12)} = 1208.3 \text{ k/in.} \qquad \frac{6EI}{L^2} = \frac{6[29(10^3)(500)]}{[20(12)]^2} = 1510.4 \text{ k}$$

$$\frac{12EI}{L^3} = \frac{12[29(10^3)(500)]}{[20(12)]^3} = 12.6 \text{ k/in.} \qquad \frac{4EI}{L} = \frac{4[29(10^3)(500)]}{20(12)} = 241.7(10^3) \text{ k} \cdot \text{in.}$$

$$\frac{2EI}{L} = \frac{2[29(10^3)(500)]}{20(12)} = 120.83(10^3) \text{ k} \cdot \text{in.}$$

#### Member 1:

$$\lambda_x = \frac{20 - 0}{20} = 1 \qquad \lambda_y = \frac{0 - 0}{20} = 0$$

#### Member 2:

$$\lambda_x = \frac{20 - 20}{20} = 0$$
  $\lambda_y = \frac{-20 - 0}{20} = -1$ 

### **Displacements and Loads.** Expanding to determine the displacements yields

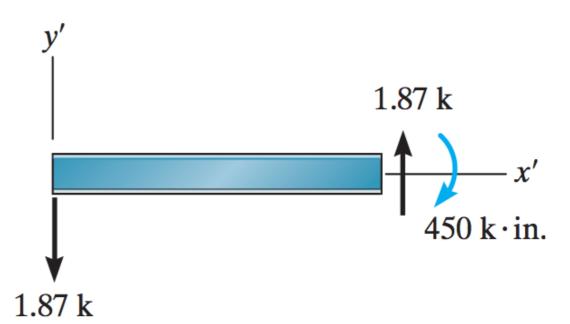
$$\begin{bmatrix} 5 \\ 0 \\ 0 \\ 0 \\ 0 \end{bmatrix} = \begin{bmatrix} 1220.9 & 0 & 1510.4 & -1208.3 & 0 \\ 0 & 1220.9 & -1510.4 & 0 & -1510.4 \\ 1510.4 & -1510.4 & 483.3(10^3) & 0 & 120.83(10^3) \\ -1208.3 & 0 & 0 & 1208.3 & 0 \\ 0 & -1510.4 & 120.83(10^3) & 0 & 241.7(10^3) \end{bmatrix} \begin{bmatrix} D_1 \\ D_2 \\ D_3 \\ D_4 \\ D_5 \end{bmatrix} + \begin{bmatrix} 0 \\ 0 \\ 0 \\ 0 \end{bmatrix}$$

$$\begin{bmatrix} D_1 \\ D_2 \\ D_3 \\ D_4 \\ D_5 \end{bmatrix} = \begin{bmatrix} 0.696 \text{ in.} \\ -1.55(10^{-3}) \text{ in.} \\ -2.488(10^{-3}) \text{ rad} \\ 0.696 \text{ in.} \\ 1.234(10^{-3}) \text{ rad} \end{bmatrix}$$

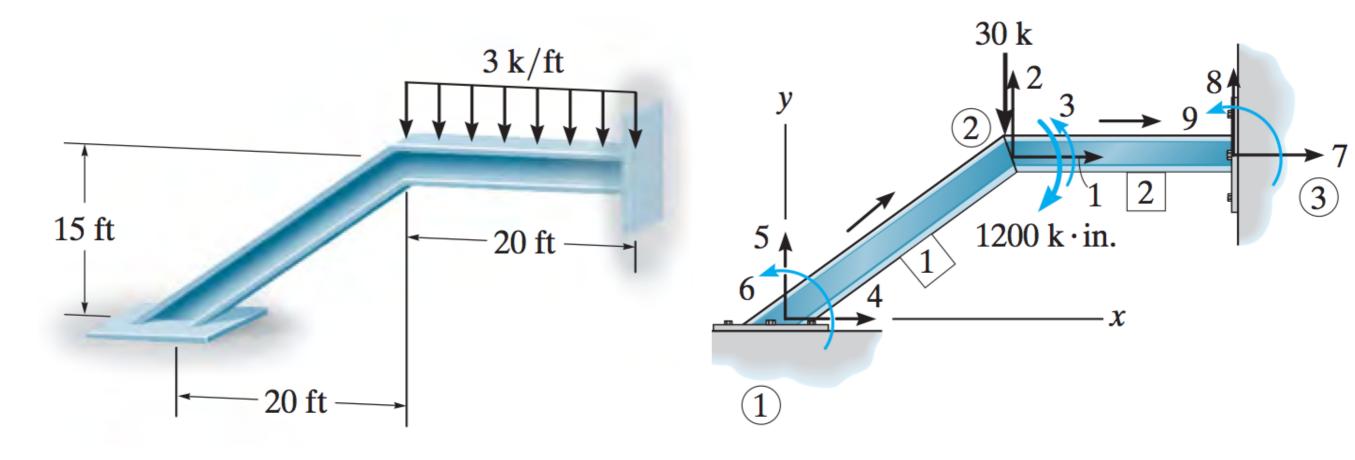
$$\begin{bmatrix} Q_6 \\ Q_7 \\ Q_8 \\ Q_9 \end{bmatrix} = \begin{bmatrix} 1 & 2 & 3 & 4 & 5 \\ 0 & -12.6 & 1510.4 & 0 & 1510.4 \\ -12.6 & 0 & -1510.4 & 0 & 0 \\ 0 & -1208.3 & 0 & 0 & 0 \\ 1510.4 & 0 & 120.83(10^3) & 0 & 0 \end{bmatrix} \begin{bmatrix} 0.696 \\ -1.55(10^{-3}) \\ -2.488(10^{-3}) \\ 0.696 \\ 1.234(10^{-3}) \end{bmatrix} + \begin{bmatrix} 0 \\ 0 \\ 0 \\ 0 \end{bmatrix} = \begin{bmatrix} -1.87 \text{ k} \\ -5.00 \text{ k} \\ 1.87 \text{ k} \\ 750 \text{ k} \cdot \text{in.} \end{bmatrix}$$

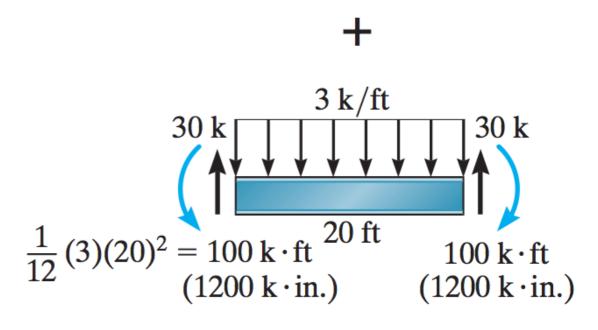
$$\mathbf{q}_1 = \mathbf{k}_1 \mathbf{T}_1 \mathbf{D} = \begin{bmatrix} 1208.3 & 0 & 0 & -1208.3 & 0 & 0 \\ 0 & 12.6 & 1510.4 & 0 & -12.6 & 1510.4 \\ 0 & 1510.4 & 241.7(10^3) & 0 & -1510.4 & 120.83(10^3) \\ 0 & -1208.3 & 0 & 0 & 1208.3 & 0 & 0 \\ 0 & 0 & 1510.4 & 120.83(10^3) & 0 & -1510.4 & 241.7(10^3) \end{bmatrix} \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix} \begin{bmatrix} 0.696 \\ 0 \\ 1.234(10^{-3}) \\ 0.696 \\ -1.55(10^{-3}) \\ -2.488(10^{-3}) \end{bmatrix} \frac{1}{3}$$

$$\begin{bmatrix} q_4 \\ q_6 \\ q_5 \\ q_1 \\ q_2 \\ q_3 \end{bmatrix} = \begin{bmatrix} 0 \\ -1.87 \text{ k} \\ 0 \\ 0 \\ 1.87 \text{ k} \\ -450 \text{ k} \cdot \text{in.} \end{bmatrix}$$



Determine the loadings at the ends of each member of the frame shown in Fig. 16–5a. Take  $I = 600 \text{ in}^4$ ,  $A = 12 \text{ in}^2$ , and  $E = 29(10^3)$  ksi for each member.





#### **Structure Stiffness Matrix**

#### Member 1:

$$\frac{AE}{L} = \frac{12[29(10^3)]}{25(12)} = 1160 \text{ k/in.}$$

$$\frac{12EI}{L^3} = \frac{12[29(10^3)]600}{[25(12)]^3} = 7.73 \text{ k/in.}$$

$$\frac{6EI}{L^2} = \frac{6[29(10^3)]600}{[25(12)]^2} = 1160 \text{ k}$$

$$\frac{4EI}{L} = \frac{4[29(10^3)]600}{25(12)} = 232(10^3) \text{ k} \cdot \text{in.}$$

$$\frac{2EI}{L} = \frac{2[29(10^3)]600}{25(12)} = 116(10^3) \text{ k} \cdot \text{in.}$$

$$\lambda_x = \frac{20 - 0}{25} = 0.8 \qquad \lambda_y = \frac{15 - 0}{25} = 0.6$$

$$\frac{AE}{L} = \frac{12[29(10^3)]}{20(12)} = 1450 \text{ k/in.}$$

$$\frac{12EI}{L^3} = \frac{12[29(10^3)]600}{[20(12)]^3} = 15.10 \text{ k/in.}$$

$$\frac{6EI}{L^2} = \frac{6[29(10^3)]600}{[20(12)]^2} = 1812.50 \text{ k}$$

$$\frac{4EI}{L} = \frac{4[29(10^3)]600}{20(12)} = 2.90(10^5) \text{ k} \cdot \text{in.}$$

$$\frac{2EI}{L} = \frac{2[29(10^3)]600}{[20(12)]} = 1.45(10^5) \text{ k} \cdot \text{in.}$$

$$\lambda_x = \frac{40 - 20}{20} = 1 \qquad \lambda_y = \frac{15 - 15}{20} = 0$$

$$\mathbf{k}_1 = \begin{bmatrix} 745.18 & 553.09 & -696 & -745.18 & -553.09 & -696 \\ 553.09 & 422.55 & 928 & -553.09 & -422.55 & 928 \\ -696 & 928 & 232(10^3) & 696 & -928 & 116(10^3) \\ -745.18 & -553.09 & 696 & 745.18 & 553.09 & 696 \\ -553.09 & -422.55 & -928 & 553.09 & 422.55 & -928 \\ -696 & 928 & 116(10^3) & 696 & -928 & 232(10^3) \end{bmatrix} \begin{bmatrix} 3 & 3 & 3 & 3 & 3 & 3 & 3 \\ 4 & 5 & 553.09 & -696 & 328 & 322(10^3) \\ -696 & 928 & 116(10^3) & 696 & -928 & 232(10^3) \end{bmatrix} \begin{bmatrix} 3 & 3 & 3 & 3 & 3 & 3 \\ 4 & 5 & 553.09 & -696 & 328 & 322(10^3) \\ -696 & 928 & 116(10^3) & 696 & -928 & 232(10^3) \end{bmatrix} \begin{bmatrix} 3 & 3 & 3 & 3 & 3 & 3 \\ 4 & 5 & 553.09 & -696 & 328 & 322(10^3) \\ -696 & 928 & 116(10^3) & 696 & -928 & 232(10^3) \end{bmatrix} \begin{bmatrix} 3 & 3 & 3 & 3 & 3 & 3 \\ 4 & 5 & 553.09 & -696 & 328 & 322(10^3) \\ -696 & 928 & 116(10^3) & 696 & -928 & 232(10^3) \end{bmatrix} \begin{bmatrix} 3 & 3 & 3 & 3 & 3 & 3 \\ 3 & 5 & 553.09 & -696 & 328 & 322(10^3) \\ -696 & 928 & 116(10^3) & 696 & -928 & 232(10^3) \end{bmatrix} \begin{bmatrix} 3 & 3 & 3 & 3 & 3 & 3 \\ 3 & 5 & 553.09 & -696 & 328 & 322(10^3) \\ -696 & 928 & 116(10^3) & 696 & -928 & 232(10^3) \end{bmatrix} \begin{bmatrix} 3 & 3 & 3 & 3 & 3 & 3 \\ 3 & 5 & 553.09 & -696 & 328 & 322(10^3) \\ -696 & 928 & 116(10^3) & 696 & -928 & 232(10^3) \end{bmatrix}$$

$$\mathbf{k}_2 = \begin{bmatrix} 1450 & 0 & 0 & -1450 & 0 & 0 \\ 0 & 15.10 & 1812.50 & 0 & -15.10 & 1812.50 \\ 0 & 1812.50 & 290(10^3) & 0 & -1812.50 & 145(10^3) \\ -1450 & 0 & 0 & 1450 & 0 & 0 \\ 0 & -15.10 & -1812.50 & 0 & 15.10 & -1812.50 \\ 0 & 1812.50 & 145(10^3) & 0 & -1812.50 & 290(10^3) \end{bmatrix}$$

Displacements and Loads. Expanding to determine the displacements, and solving, we have

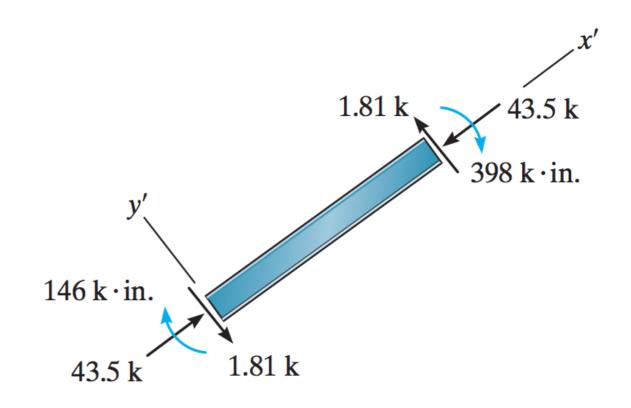
$$\begin{bmatrix} 0 \\ -30 \\ -1200 \end{bmatrix} = \begin{bmatrix} 2195.18 & 553.09 & 696 \\ 553.09 & 437.65 & 884.5 \\ 696 & 884.5 & 522(10^3) \end{bmatrix} \begin{bmatrix} D_1 \\ D_2 \\ D_3 \end{bmatrix} + \begin{bmatrix} 0 \\ 0 \\ 0 \end{bmatrix}$$

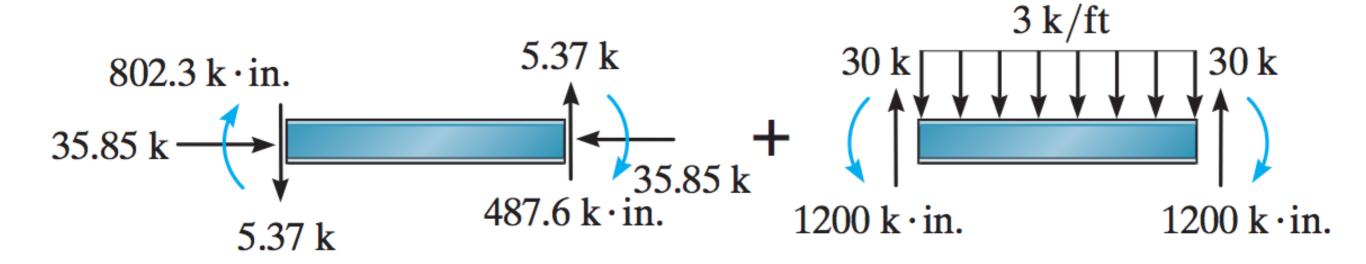
$$\begin{bmatrix} D_1 \\ D_2 \\ D_3 \end{bmatrix} = \begin{bmatrix} 0.0247 \text{ in.} \\ -0.0954 \text{ in.} \\ -0.00217 \text{ rad.} \end{bmatrix}$$

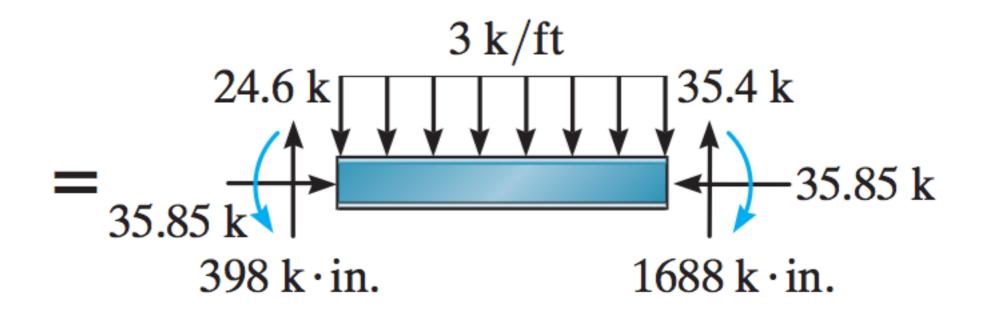
$$\begin{bmatrix} Q_4 \\ Q_5 \\ Q_6 \\ Q_7 \\ Q_8 \\ Q_9 \end{bmatrix} = \begin{bmatrix} -745.18 & -553.09 & -696 \\ -553.09 & -422.55 & 928 \\ 696 & -928 & 116(10^3) \\ -1450 & 0 & 0 \\ 0 & -15.10 & -1812.50 \\ 0 & 1812.50 & 145(10^3) \end{bmatrix} \begin{bmatrix} 0.0247 \\ -0.0954 \\ -0.00217 \end{bmatrix} + \begin{bmatrix} 0 \\ 0 \\ 0 \\ 0 \\ 0 \end{bmatrix} = \begin{bmatrix} 35.85 \text{ k} \\ 24.63 \text{ k} \\ -145.99 \text{ k} \cdot \text{in.} \\ -35.85 \text{ k} \\ 5.37 \text{ k} \\ -487.60 \text{ k} \cdot \text{in.} \end{bmatrix}$$

$$\begin{bmatrix} q_4 \\ q_5 \\ q_6 \\ q_1 \\ q_2 \\ q_3 \end{bmatrix} = \begin{bmatrix} 1160 & 0 & 0 & -1160 & 0 & 0 \\ 0 & 7.73 & 1160 & 0 & -7.73 & 1160 \\ 0 & 1160 & 232(10^3) & 0 & -1160 & 116(10^3) \\ 0 & 1160 & 0 & 0 & 1160 & 0 & 0 \\ 0 & -7.73 & -1160 & 0 & 7.73 & -1160 \\ 0 & 1160 & 116(10^3) & 0 & -1160 & 232(10^3) \end{bmatrix} \begin{bmatrix} 0.8 & 0.6 & 0 & 0 & 0 & 0 \\ -0.6 & 0.8 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0.8 & 0.6 & 0 \\ 0 & 0 & 0 & 0 & 0.8 & 0.6 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0.8 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix} \begin{bmatrix} 0 \\ 0 \\ 0 \\ 0.0247 \\ -0.0954 \\ -0.00217 \end{bmatrix} 3$$

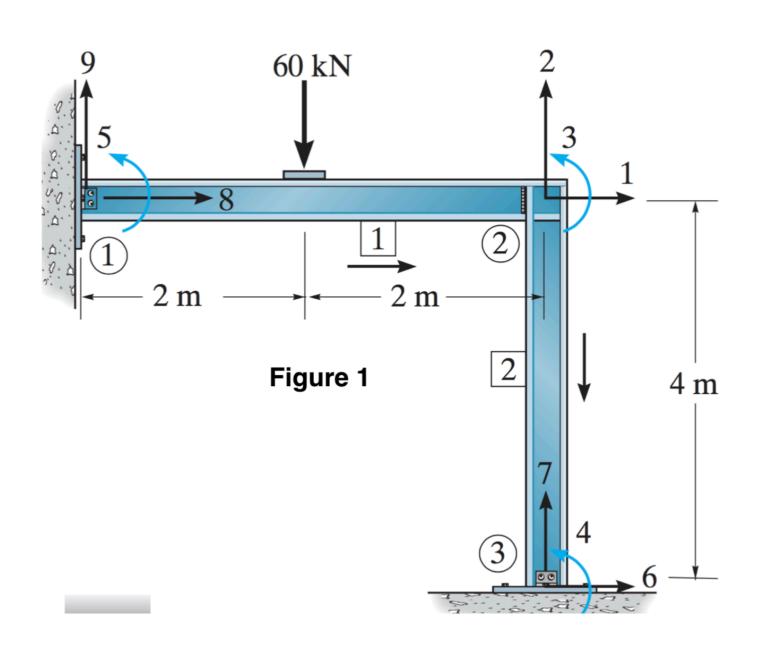
$$\begin{bmatrix} q_4 \\ q_5 \\ q_6 \\ q_1 \\ q_2 \\ q_3 \end{bmatrix} = \begin{bmatrix} 43.5 \text{ k} \\ -1.81 \text{ k} \\ -146 \text{ k} \cdot \text{in.} \\ -43.5 \text{ k} \\ 1.81 \text{ k} \\ -398 \text{ k} \cdot \text{in.} \end{bmatrix}$$







Determine the support reactions at pins ① and ③. Take E = 200 GPa,  $I = 350(10^6)$  mm<sup>4</sup>,  $A = 15(10^3)$  mm<sup>2</sup> for each member.



**Member Stiffness Matrices.** The origin of the global coordinate system will be set at joint ①. For member  $\boxed{1}$  and  $\boxed{2}$ , L=4m.

$$\frac{AE}{L} = \frac{0.015[200(10^9)]}{4} = 750(10^6) \text{ N/m}$$

$$\frac{12EI}{L^3} = \frac{12[200(10^9)][350(10^{-6})]}{4^3} = 13.125(10^6) \text{ N/m}$$

$$\frac{6EI}{L^2} = \frac{4[200(10^9)][350(10^{-6})]}{4^2} = 26.25(10^6) \text{ N}$$

$$\frac{4EI}{L} = \frac{4[200(10^9)][350(10^{-6})]}{4} = 70(10^6) \,\mathrm{N} \cdot \mathrm{m}$$

$$\frac{2EI}{L} = \frac{2[200(10^9)][350(10^{-6})]}{4} = 35(10^6) \text{ N} \cdot \text{m}$$

For member 
$$\boxed{1}$$
,  $\lambda_x = \frac{4-0}{4} = 1$  and  $\lambda_y = \frac{0-0}{4} = 0$ . Thus, 
$$\mathbf{k}_1 = \begin{bmatrix} 8 & 9 & 5 & 1 & 2 & 3 \\ 750 & 0 & 0 & -750 & 0 & 0 \\ 0 & 13.125 & 26.25 & 0 & -13.125 & 26.25 \\ 0 & 26.25 & 70 & 0 & -26.25 & 35 \\ -750 & 0 & 0 & 750 & 0 & 0 \\ 0 & -13.125 & -26.25 & 0 & 13.125 & -26.25 \\ 0 & 26.25 & 35 & 0 & -26.25 & 70 \end{bmatrix} \begin{bmatrix} 8 \\ 9 \\ 5 \\ 1 \end{bmatrix} (10^6)$$

For member 
$$\boxed{2}$$
,  $\lambda_x = \frac{4-4}{4} = 0$ , and  $\lambda_y = \frac{-4-0}{4} = -1$ . Thus, 
$$\mathbf{k}_2 = \begin{bmatrix} 1 & 2 & 3 & 6 & 7 & 4 \\ 13.125 & 0 & 26.25 & -13.125 & 0 & 26.25 \\ 0 & 750 & 0 & 0 & -750 & 0 \\ 26.25 & 0 & 70 & -26.25 & 0 & 35 \\ -13.125 & 0 & -26.25 & 13.125 & 0 & -26.25 \\ 0 & -750 & 0 & 0 & 750 & 0 \\ 26.25 & 0 & 35 & -26.25 & 0 & 70 \end{bmatrix}$$

Structure Stiffness Matrix. It is a  $9 \times 9$  matrix since the highest code number is 9. Thus

$$\mathbf{K} = \begin{bmatrix} 1 & 2 & 3 & 4 & 5 & 6 & 7 & 8 & 9 \\ 763.125 & 0 & 26.25 & 26.25 & 0 & -13.125 & 0 & -750 & 0 \\ 0 & 763.125 & -26.25 & 0 & -26.25 & 0 & -750 & 0 & -13.125 \\ 26.25 & -26.25 & 140 & 35 & 35 & -26.25 & 0 & 0 & 26.25 \\ 26.25 & 0 & 35 & 70 & 0 & -26.25 & 0 & 0 & 0 \\ 0 & -26.25 & 35 & 0 & 70 & 0 & 0 & 0 & 26.25 \\ -13.125 & 0 & -26.25 & -26.25 & 0 & 13.125 & 0 & 0 & 0 \\ 0 & -750 & 0 & 0 & 0 & 0 & 750 & 0 & 0 \\ 0 & -13.125 & 26.25 & 0 & 26.25 & 0 & 0 & 0 & 13.125 \end{bmatrix} \begin{bmatrix} 1 \\ 2 \\ 3 \\ 4 \\ 5 \end{bmatrix}$$

**Known Nodal Loads and Deflections.** The nodal load acting on the unconstrained degree of freedom (code numbers 1, 2, 3, 4, and 5) are shown in Fig. *a* and Fig. *b*.

$$\mathbf{Q}_{k} = \begin{bmatrix} 0 \\ -41.25(10^{3}) \\ 45(10^{3}) \\ 0 \\ 0 \end{bmatrix} \begin{bmatrix} 1 \\ 2 \\ 3 \text{ and } \mathbf{D}_{k} = \begin{bmatrix} 0 \\ 0 \\ 0 \\ 0 \end{bmatrix} \begin{bmatrix} 6 \\ 7 \\ 8 \\ 9 \end{bmatrix}$$

#### Loads-Displacement Relation. Applying Q = KD,

0 -		763.125	0	26.25	26.25	0	-13.125	0	-750	0 -		$\lceil D_1 \rceil$	
$-41.25(10^3)$		0	763.125	-26.25	0	-26.25	0	-750	0	-13.125		$D_2$	
$45(10^3)$		26.25	-26.25	140	35	35	-26.25	0	0	26.25		$D_3$	
0		26.25	0	35	70	0	-26.25	0	0	0		$D_4$	
0	=	0	-26.25	35	0	70	0	0	0	26.25	$(10^6)$	$D_5$	
$Q_6$		-13.125	0	-26.25	-26.25	0	13.125	0	0	0		0	
$Q_7$		0	-750	0	0	0	0	750	0	0		0	
$Q_8$		-750	0	0	0	0	0	0	750	0		0	
$Q_9$		0	-13.125	26.25	0	26.25	0	0	0	13.125 _		L 0 ]	

From the matrix partition,  $\mathbf{Q}_k = \mathbf{K}_{11}\mathbf{D}_u + \mathbf{K}_{12}\mathbf{D}_k$ ,

$$0 = (763.125D_1 + 26.25D_3 + 26.25D_4)(10^6) \tag{1}$$

$$-41.25(10^3) = (763.125D_2 - 26.25D_3 - 26.25D_5)(10^6)$$
 (2)

$$45(10^3) = (26.25D_1 - 26.25D_2 + 140D_3 + 35D_4 + 35D_5)(10^6)$$
 (3)

$$0 = (26.25D_1 + 35D_3 + 70D_4)(10^6) (4)$$

$$0 = (-26.25D_2 + 35D_3 + 70D_5)(10^6) \tag{5}$$

Solving Eqs. (1) to (5)

$$D_1 = -7.3802(10^{-6})$$
  $D_2 = -47.3802(10^{-6})$   $D_3 = 423.5714(10^{-6})$ 

$$D_4 = -209.0181(10^{-6})$$
  $D_5 = -229.5533(10^{-6})$ 

Using these results and applying  $\mathbf{Q}_u = \mathbf{K}_{21}\mathbf{D}_u + \mathbf{K}_{22}\mathbf{D}_k$ ,

$$Q_6 = (-13.125)(10^6) - 7.3802(10^{-6}) - 26.25(10^6)423.5714(10^{-6}) - 26.25(10^6) - 209.0181(10^{-6}) + 0 = -5.535 \text{ kN}$$

$$Q_7 = -750(10^6) - 47.3802(10^{-6}) + 0 = 35.535 \text{ kN}$$

$$Q_8 = -750(10^6) - 7.3802(10^{-6}) + 0 = 5.535 \text{ kN}$$

$$Q_9 = -13.125(10^6) - 47.3802(10^{-6}) + 26.25(10^6) + 423.5714(10^{-6}) + 26.25(10^6) - 229.5533(10^{-6}) + 0 = 5.715 \text{ kN}$$

Superposition these results to those of FEM shown in Fig. a,

$$R_6 = -5.535 \text{ kN} + 0 = 5.54 \text{ kN}$$

$$R_7 = 35.535 + 0 = 35.5 \text{ kN}$$

$$R_8 = 5.535 + 0 = 5.54 \text{ kN}$$

$$R_9 = 5.715 + 18.75 = 24.5 \text{ kN}$$

Ans.

Ans.

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Ans.

