#### DEPARTMENT OF CIVIL ENGINEERING CE-221 SOLID MECHANICS

#### **Mid-Sem Exam**

#### 06/09/16

PAPER CODE: A

**Note:** Write your name & roll no. on answerbook and on summary-answer-sheet provided with the question paper. **You must submit the summary-answer-sheet along with the answerbook.** 

Closed book, closed notes exam. No formula sheet allowed. No mobile phones allowed in the exam hall.

All questions carry equal marks. Assume suitable data if required and state the same clearly.

Use formulae from provided tables, **<u>if applicable</u>**.

#### Problem 1

A symmetric bimetallic bar consisting of an inner copper layer securely bonded to two outer steel layers (**Fig. 1**) is heated uniformly to increase its temperature by **100** °C. Assuming that the width of the bar is **30 mm**, the length is **500 mm**, and the thickness of each of the three layers is **10 mm**, determine the normal stresses  $\sigma_s$  and  $\sigma_c$  in steel and copper, respectively. Coefficient of thermal expansion of steel is **18 x 10**<sup>-6</sup> /°C and copper is **25 x 10**<sup>-6</sup> /°C. Modulus of elasticity of steel is **190 GPa** and copper is **75 GPa**.



Fig. 1

#### **Problem 2**

The movement of a 100 mm x 50 mm plate is restrained along the x-direction by supports along two edges as shown in Fig. 2. There is no restraint to the movement of the plate in the y and z directions at the supports. The coefficient of thermal expansion is 25 x  $10^{-6}$ /°C, modulus of elasticity is 200 GPa, and Poisson's ratio is 0.25. Calculate the change in length of the plate along the y direction ( $\delta_v$ ) due to a temperature change of 100 °C.

<u>Note</u> that the normal strains in the plate along any direction is the summation of strains due to mechanical stresses and thermal changes, i.e.  $\varepsilon_x = \sigma_x/E - \nu\sigma_y/E - \nu\sigma_z/E + \alpha\Delta T$  and similarly for  $\varepsilon_y$  and  $\varepsilon_z$ .



## Problem 3

A system of two concentric torsion members connected at their ends by rigid disks G is shown in Fig. 3. The outside member C is a tube with outside diameter 300 mm and thickness 10 mm. The inside member consists of solid shafts A of diameter 100 mm and B of diameter 200 mm. Shafts A and B and tube C are made of the same material having shear modulus 100 GPa. At the right end the rigid disk is fixed in support D. A torque of 200 kNm is applied to the outside tube as shown in Fig. 3. Determine the maximum shear stress tube in and the rotation of the left end rigid disk С  $(\tau_{C.max})$  $(\phi_G)$ .



Fig. 3

## Problem 4

Draw the shear force and bending moment diagrams of the 8 m long lower continuous beam that has an internal hinge/pin as shown in **Fig. 4**. The BMD and SFD for the upper simply supported beam is not required.



## SUMMARY ANSWER SHEET

## PAPER CODE: A

Name:

Roll no:

## Problem 1

 $\sigma_s$  =

 $\sigma_c =$ 

## Problem 2

 $\delta_y =$ 

## Problem 3

 $\tau_{C,max} =$ 

 $\phi_G =$ 

#### DEPARTMENT OF CIVIL ENGINEERING CE-221 SOLID MECHANICS 06/09/16

#### **PAPER CODE: B**

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#### Problem 1

Mid-Sem Exam

A symmetric bimetallic bar consisting of an inner copper layer securely bonded to two outer steel layers (Fig. 1) is heated uniformly to increase its temperature by 150 °C. Assuming that the width of the bar is 35 mm, the length is 600 mm, and the thickness of each of the three layers is 15 mm, determine the normal stresses  $\sigma_s$  and  $\sigma_c$  in steel and copper, respectively. Coefficient of thermal expansion of steel is 22 x 10<sup>-6</sup> /°C and copper is 28 x 10<sup>-6</sup> /°C. Modulus of elasticity of steel is 195 GPa and copper is 80 GPa.



Fig. 1

#### Problem 2

The movement of a 150 mm x 75 mm plate is restrained along the x-direction by supports along two edges as shown in Fig. 2. There is no restraint to the movement of the plate in the y and z directions at the supports. The coefficient of thermal expansion is 30 x  $10^{-6}$ /°C, modulus of elasticity is 210 GPa, and Poisson's ratio is 0.30. Calculate the change in length of the plate along the y direction ( $\delta_v$ ) due to a temperature change of 125 °C.

<u>Note</u> that the normal strains in the plate along any direction is the summation of strains due to mechanical stresses and thermal changes, i.e.  $\varepsilon_x = \sigma_x/E - \nu\sigma_y/E - \nu\sigma_z/E + \alpha\Delta T$  and similarly for  $\varepsilon_y$  and  $\varepsilon_z$ .



Problem 3

A system of two concentric torsion members connected at their ends by rigid disks G is shown in Fig. 3. The outside member C is a tube with outside diameter 400 mm and thickness 15 mm. The inside member consists of solid shafts A of diameter 125 mm and Bof diameter 250 mm. Shafts A and B and tube C are made of the same material having shear modulus 110 GPa. At the right end the rigid disk is fixed in support D. A torque of 250 kNm is applied to the outside tube as shown in Fig. 3. Determine the maximum shear stress in tube C ( $\tau_{C,max}$ ) and the rotation of the left end rigid disk ( $\phi_G$ ).



Fig. 3

#### **Problem 4**

Draw the shear force and bending moment diagrams of the 12 m long lower continuous beam that has an internal hinge/pin as shown in **Fig. 4**. The BMD and SFD for the upper simply supported beam is not required.



### PAPER CODE: B

#### Name:

Roll no:

## Problem 1

 $\sigma_s =$ 

 $\sigma_c =$ 

# Problem 2

 $\delta_y =$ 

## Problem 3

 $au_{A,max} =$   $au_{B,max} =$   $au_{C,max} =$   $\phi_G =$ 

#### DEPARTMENT OF CIVIL ENGINEERING CE-221 SOLID MECHANICS 06/09/16

#### Mid-Sem Exam

PAPER CODE: C

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### Problem 1

A symmetric bimetallic bar consisting of an inner copper layer securely bonded to two outer steel layers (**Fig. 1**) is heated uniformly to increase its temperature by **200** °C. Assuming that the width of the bar is **40 mm**, the length is **700 mm**, and the thickness of each of the three layers is **20 mm**, determine the normal stresses  $\sigma_s$  and  $\sigma_c$  in steel and copper, respectively. Coefficient of thermal expansion of steel is **26 x 10**<sup>-6</sup> /°C and copper is **32 x 10**<sup>-6</sup> /°C. Modulus of elasticity of steel is **200 GPa** and copper is **85 GPa**.



Fig. 1

## Problem 2

The movement of a 200 mm x 100 mm plate is restrained along the x-direction by supports along two edges as shown in Fig. 2. There is no restraint to the movement of the plate in the y and z directions at the supports. The coefficient of thermal expansion is 35 x  $10^{-6}$ /°C, modulus of elasticity is 220 GPa, and Poisson's ratio is 0.35. Calculate the change in length of the plate along the y direction ( $\delta_v$ ) due to a temperature change of 150 °C.

<u>Note</u> that the normal strains in the plate along any direction is the summation of strains due to mechanical stresses and thermal changes, i.e.  $\varepsilon_x = \sigma_x/E - \nu\sigma_y/E - \nu\sigma_z/E + \alpha\Delta T$  and similarly for  $\varepsilon_y$  and  $\varepsilon_z$ .



## Problem 3

A system of two concentric torsion members connected at their ends by rigid disks G is shown in Fig. 3. The outside member C is a tube with outside diameter 500 mm and thickness 20 mm. The inside member consists of solid shafts A of diameter 150 mm and Bof diameter 300 mm. Shafts A and B and tube C are made of the same material having shear modulus 120 GPa. At the right end the rigid disk is fixed in support D. A torque of 300 kNm is applied to the outside tube as shown in Fig. 3. Determine the maximum shear stress in tube C ( $\tau_{C,max}$ ) and the rotation of the left end rigid disk ( $\phi_G$ ).



Fig. 3

## Problem 4

Draw the shear force and bending moment diagrams of the 16 m long lower continuous beam that has an internal hinge/pin as shown in **Fig. 4**. The BMD and SFD for the upper simply supported beam is not required.



Fig. 4

#### SUMMARY ANSWER SHEET

## PAPER CODE: C

Name:

#### Roll no:

## Problem 1

 $\sigma_s =$ 

 $\sigma_c =$ 

## Problem 2

 $\delta_{y} =$ 

## Problem 3

 $\tau_{A,max} =$  $\tau_{B,max} =$  $\tau_{C,max} =$  $\phi_G =$ 

#### DEPARTMENT OF CIVIL ENGINEERING CE-221 SOLID MECHANICS 06/09/16

#### Mid-Sem Exam

PAPER CODE: D

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## Problem 1

A symmetric bimetallic bar consisting of an inner copper layer securely bonded to two outer steel layers (**Fig. 1**) is heated uniformly to increase its temperature by **250** °C. Assuming that the width of the bar is **45 mm**, the length is **800 mm**, and the thickness of each of the three layers is **25 mm**, determine the normal stresses  $\sigma_s$  and  $\sigma_c$  in steel and copper, respectively. Coefficient of thermal expansion of steel is **29 x 10**<sup>-6</sup> /°C and copper is **35 x 10**<sup>-6</sup> /°C. Modulus of elasticity of steel is **205 GPa** and copper is **90 GPa**.





#### Problem 2

The movement of a 250 mm x 125 mm plate is restrained along the x-direction by supports along two edges as shown in Fig. 2. There is no restraint to the movement of the plate in the y and z directions at the supports. The coefficient of thermal expansion is 40 x  $10^{-6}$ /°C, modulus of elasticity is 230 GPa, and Poisson's ratio is 0.4. Calculate the change in length of the plate along the y direction ( $\delta_y$ ) due to a temperature change of 175 °C.

<u>Note</u> that the normal strains in the plate along any direction is the summation of strains due to mechanical stresses and thermal changes, i.e.  $\varepsilon_x = \sigma_x/E - \nu\sigma_y/E - \nu\sigma_z/E + \alpha\Delta T$  and similarly for  $\varepsilon_y$  and  $\varepsilon_z$ .



## Problem 3

A system of two concentric torsion members connected at their ends by rigid disks G is shown in Fig. 3. The outside member C is a tube with outside diameter 600 mm and thickness 25 mm. The inside member consists of solid shafts A of diameter 175 mm and Bof diameter 350 mm. Shafts A and B and tube C are made of the same material having shear modulus 130 GPa. At the right end the rigid disk is fixed in support D. A torque of 350 kNm is applied to the outside tube as shown in Fig. 3. Determine the maximum shear stress in tube C ( $\tau_{C,max}$ ) and the rotation of the left end rigid disk ( $\phi_G$ ).



Fig. 3

## Problem 4

Draw the shear force and bending moment diagrams of the 20 m long lower continuous beam that has an internal hinge/pin as shown in **Fig. 4**. The BMD and SFD for the upper simply supported beam is not required.



Fig. 4

#### SUMMARY ANSWER SHEET

## PAPER CODE: D

Name:

#### Roll no:

## Problem 1

 $\sigma_s =$ 

 $\sigma_c =$ 

## Problem 2

 $\delta_{y} =$ 

## Problem 3

 $au_{A,max} =$   $au_{B,max} =$   $au_{C,max} =$   $\phi_G =$ 

CE221 2016 1 M.DSEM.  $\frac{P_1}{\overbrace{s}} \xrightarrow{P_s/2} F_{s/2} \qquad F_$ Compat:  $\delta_s = \delta_c$  (-ls) ⇒ x ATL + PsL = x ATL + PcL  $A_s = 2A_c = 2A$ .  $\overline{U_{S}} = \frac{P_{S}}{2A} = \left( \frac{(\alpha_{c} - \alpha_{s})\Delta T}{\frac{1}{E_{s}} + \frac{2}{E_{c}}} \right) ; \quad \overline{U_{c}} = \frac{P_{c}}{A} = -\frac{P_{s}}{A} = -2\overline{U_{s}}$ ZAES  $(: \alpha_c > \alpha_s, T_s(T), T_c(C), as expected).$ Codes:  $A \rightarrow \overline{r_{5}} = 21.92$ ,  $\overline{r_{c}} = -43.85$   $B \rightarrow 29.87$ , -59.74 MPa  $C \rightarrow 42.06$ , -84.12  $D \rightarrow 55.35$ , -110.70 $\frac{P2}{E_{x}} = 0, \quad T_{y} = T_{z} = 0 \quad (:: constrained in x-direction)$  $dy = E_y \cdot L_y$  $E_y = \frac{\pi}{y/E} - \nu \frac{\pi}{x/E} - \nu \frac{\pi}{E/E} + \alpha \Delta T$  $\varepsilon_{\mathbf{X}} = \nabla_{\mathbf{X}/\mathbf{E}} - \nu \nabla_{\mathbf{Y}/\mathbf{E}} - \nu \nabla_{\mathbf{E}/\mathbf{E}} + \kappa \Delta T$  $\Rightarrow \epsilon_y = -\nu(-\epsilon_x \Delta T)/\epsilon + x \Delta T = x \Delta T (1+\nu)$  $\delta_y = \alpha \Delta \overline{r} (1 + \nu) Ly$ . Gde A: Sy = 0.156250 7 A: 09 = 0.365625 B: = 0.365625 mm. C: = 0.708750 mm. D := 1.225

P3. Let L, L, denote lengths of hollow shaft to the left & right of applied turgne, respectively, and TC, & TCZ denote corresponding internal torques in C. Int. torque in ARB is TA=TB. (TC1/2 shown for convenience actually it's TC1 in hollow shaft C). Equil: left disk =>> T<sub>A</sub> G T<sub>C1/2</sub>  $T_A + T_{C_1} = 0$ holloo holloo shaft  $T_{C1} \longrightarrow T_{C2}$ Compatibility: at left disk G,  $p_A = p_C$   $T_{C1} = T + T_{C2}$   $T_{C2} = T_{C1} - T used$  $T_{A}\left(\begin{array}{c}L_{A} + L_{B}\\ (GJ)_{A} (GJ)_{B}\end{array}\right) = \frac{T_{C_{1}}L_{1} + (T_{C_{1}} - T)L_{2}}{(GJ)_{C}}$  $\Rightarrow \boxed{T_{CI}} = \underbrace{TL_2}_{(GJ)_C} \left[ \underbrace{L_A}_{(GJ)_B} + \underbrace{L_B}_{(GJ)_C} + \underbrace{L_i}_{(GJ)_C} + \underbrace{L_i}_{(GJ)$  $(T_c)_{max} = \max \left[ T_{c1}, T_{c2} \right].$ For given dimensions,  $|T_{c2}| > |T_{c1}|$ , so  $(T_c)_{max} = T_{c2}$   $[(T_c)_{max} = T_{c2} = \frac{T_{c2} T_{o,c}}{T}, where T_{c2} = T_{c1} - T]$  $J_{A} = \frac{1}{32} (d_{A})^{4}, \ J_{B} = \frac{1}{32} (d_{B})^{4}, \ J_{C} = \frac{1}{32} (d_{0,C} - d_{i,C}).$ 

 $T_{c2} = -190.9153$ ,  $(T_c)_{max} = 149.32$ ,  $Q_6 = 5.78$ Wdes: A -> TC1 = 9-0817, = 8.1131 , =-241.8869, = 2.56 =71-85, =7-9101, (IC) max, =-192.0899, = 41-96 , =1.38 = 8.0127,=-341.9873, =27.43, =-833 KN.m. RN.m ×10-3 MPa QG / If you do with physically correct directions,  $T_{c_1/2} = T_{c_1} = T_$ Compatibility:  $\phi_A = -\phi_c$  (: rotations in opp directions as per FBD's)  $T_{A}\left[\frac{L_{A}}{(GJ)_{A}}+\frac{L_{B}}{(GJ)_{E}}\right] = -\left[\begin{array}{c}T_{C_{1}}L_{1}\\(GJ)_{e}\end{array}-\frac{T_{C_{2}}L_{2}}{(GJ)_{e}}\right] \ll after substituting these you get these you get some result as an event of a previous Pg.$ If you do by equivalent springs,  $\begin{array}{c} F = \left[ \begin{array}{c} F = \left[ \end{array}{c} F = \left[ \begin{array}{c} F = \left[ \begin{array}{c} F = \left[ \end{array}{c} F = \left[ \begin{array}{c} F = \left[ \begin{array}{c} F = \left[ \end{array}{c} F = \left[ \end{array}{c} F = \left[ \begin{array}{c} F = \left[ \end{array}{c} F = \left[ \end{array}{c$  $\begin{array}{c} T_{i} \\ T_{i} = -T_{A} \\ T_{A} \\ equil. \end{array}$ TA = (effective stiffness of love springs) \* Qa  $T_A = \left[ \begin{array}{c} \hline G_{J/L} \\ \hline G_{J/L} \\ A \end{array} \right]^{-1} \left[ \begin{array}{c} \hline G_{J/L} \\ \hline B \\ \hline \end{array} \right]^{-1} \left[ \begin{array}{c} \hline G_{J/L} \\ \hline B \\ \hline \end{array} \right]^{-1} \left[ \begin{array}{c} \hline G_{J/L} \\ \hline \end{array} \right]_{B} \end{array} \right]^{-1} \left[ \begin{array}{c} \hline G_{J/L} \\ \hline \end{array} \right]_{B} \\ \hline \end{array} \right]^{-1} \left[ \begin{array}{c} \hline G_{J/L} \\ \hline \end{array} \right]_{B} \\ \hline \end{array} \right]^{-1} \left[ \begin{array}{c} \hline G_{J/L} \\ \hline \end{array} \right]_{B} \\ \hline \end{array} \right]^{-1} \left[ \begin{array}{c} \hline G_{J/L} \\ \hline \end{array} \right]_{B} \\ \hline \end{array} \right]^{-1} \left[ \begin{array}{c} \hline G_{J/L} \\ \hline \end{array} \right]_{B} \\ \hline \end{array} \right]^{-1} \left[ \begin{array}{c} \hline G_{J/L} \\ \hline \end{array} \right]_{B} \\ \hline \end{array} \right]^{-1} \left[ \begin{array}{c} \hline G_{J/L} \\ \hline \end{array} \right]_{B} \\ \hline \end{array} \right]^{-1} \left[ \begin{array}{c} \hline G_{J/L} \\ \hline \end{array} \right]_{B} \\ \hline \end{array} \right]^{-1} \left[ \begin{array}{c} \hline G_{J/L} \\ \hline \end{array} \right]_{B} \\ \hline \end{array} \right]^{-1} \left[ \begin{array}{c} \hline G_{J/L} \\ \hline \end{array} \right]_{B} \\ \hline \end{array} \right]^{-1} \left[ \begin{array}{c} \hline G_{J/L} \\ \hline \end{array} \right]_{B} \\ \hline \end{array} \right]^{-1} \left[ \begin{array}{c} \hline G_{J/L} \\ \hline \end{array} \right]_{B} \\ \hline \end{array} \right]^{-1} \left[ \begin{array}{c} \hline G_{J/L} \\ \hline \end{array} \right]_{B} \\ \hline \end{array} \right]^{-1} \left[ \begin{array}{c} \hline G_{J/L} \\ \hline \end{array} \right]_{B} \\ \hline \end{array} \right]^{-1} \left[ \begin{array}{c} \hline G_{J/L} \\ \hline \end{array} \right]_{B} \\ \hline \end{array} \right]^{-1} \left[ \begin{array}{c} \hline G_{J/L} \\ \hline \end{array} \right]_{B} \\ \hline \end{array} \right]^{-1} \left[ \begin{array}{c} \hline G_{J/L} \\ \hline \end{array} \right]_{B} \\ \hline \end{array} \right]^{-1} \left[ \begin{array}{c} \hline G_{J/L} \\ \hline \end{array} \right]_{B} \\ \hline \end{array} \right]^{-1} \left[ \begin{array}{c} \hline G_{J/L} \\ \hline \end{array} \right]_{B} \\ \hline \end{array} \right]^{-1} \left[ \begin{array}{c} \hline G_{J/L} \\ \hline \end{array} \right]_{B} \\ \hline \end{array} \right]^{-1} \left[ \begin{array}{c} \hline G_{J/L} \\ \hline \end{array} \right]_{B} \\ \hline \end{array} \right]^{-1} \left[ \begin{array}{c} \hline G_{J/L} \\ \hline \\ \end{array} \right]_{B} \\ \hline \end{array} \right]^{-1} \left[ \begin{array}{c} \hline G_{J/L} \\ \\ \end{array} \right]_{B} \\ \hline \end{array} \right]^{-1} \left[ \begin{array}{c} \hline G_{J/L} \\ \\ \end{array} \right]_{B} \\ \hline \end{array} \right]^{-1} \left[ \begin{array}{c} \hline G_{J/L} \\ \\ \end{array} \right]_{B} \\ \hline \end{array} \right]^{-1} \left[ \begin{array}{c} \hline G_{J/L} \\ \\ \end{array} \right]_{B} \\ \hline \\ \end{array}$ Also, Qa = extension of left upper spring + ext of right upper  $Q_G = \frac{T_i}{GJ_c/L_i} + \frac{T_i - T}{GJ_c/L_2}$ . Eliminate for and TA and get Ti same as Tc, in (A) on previous pg.

A = AB = AA = RA RB = RA RB = RA RB = 2 2 2hinge, so only  $\Sigma M_A = 0 \Rightarrow R_B = \frac{Wa}{2}(3a) + Wa(\frac{5}{2}a) + \frac{P}{2}(2a)$ internal shear Wa/2, no bending moment. a = 4wa + PP/2+3/2Wa, 3wq (+) wa/2 < starting from right end proceeding leftward using V'=-W. -wa/z Values, left to right are: (-) Code A: -35, 25, 15, 5, -5 B: -71, 47, 36, 12, -12 -P-SWa C: -112, 72, 60, 20, -20 ala D: -163, 103, 90, 30, -30 K Wa2 E starting from left end -wa 1 guadratic. proceeding rightward straight line Values, left to right are: -(P#5wa)a Gode A: - 70, -20, 2.5 Kink. Slope before B: -213, -72, 9kink is P+3wa, C = -448, -160, 20slope after kink D = - 875, -300, 37.5 is swa.