

# DEPARTMENT OF CIVIL ENGINEERING, IIT BOMBAY

## CE 221 Solid Mechanics: QUIZ 1

Note: Assume suitable data if not given.

22/08/2015

Total Marks:10

Duration: 60 mins

Instructors A. Laskar / N.K.Chandiramani

### Problem 1 (5 marks):

A composite bar of square cross-section is constructed of two different materials having moduli of elasticity  $E_1$  and  $E_2$  as shown in Fig. 1. Both parts of the bar have the same cross-sectional dimensions (i.e.,  $2b \times b$ ). The bar is loaded with a compressive force  $P$  acting at an eccentricity  $e$  measured from the interface of the two materials, as shown. Assuming that the end plates are rigid:

- (a) Derive a formula for the eccentricity  $e$  of the load  $P$  so that each part of the bar is stressed uniformly in compression.
- (b) Under these conditions what part of the load  $P$  does each material carry?

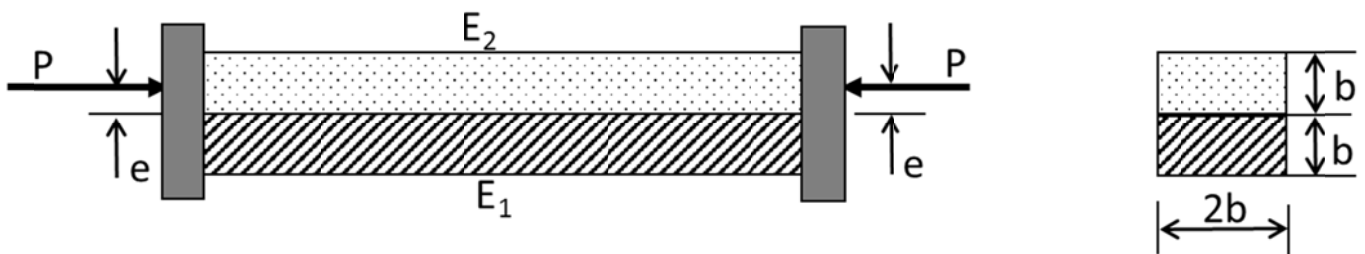


Fig. 1

### Problem 2 (5 marks):

A piece of  $50 \text{ mm} \times 250 \text{ mm} \times 10 \text{ mm}$  steel plate is subjected to uniformly distributed stresses along its edges as shown in Fig. 2. If  $P_y = 2P_x$  and the maximum shear stress acting on the plate under the given loading is  $60 \text{ MPa}$ , determine:

- (a) the applied load  $P_x$ ,
  - (b) the change in thickness of the plate under the applied loads,
- Assume  $E = 200 \text{ GPa}$  and  $\nu = 0.25$ .

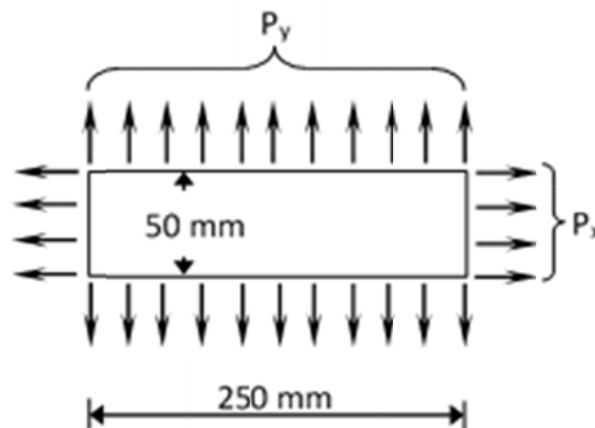
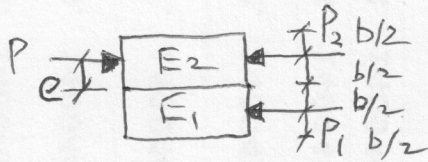


Fig. 2

P.1



Equilibrium:

$$P = P_1 + P_2 \rightarrow \textcircled{1}$$

$$P(e+b) = P_1 \frac{b}{2} + P_2 \frac{3b}{2} \rightarrow \textcircled{2}$$

Compatibility: axial strain  $\epsilon$  same in  $E_1, E_2$ .

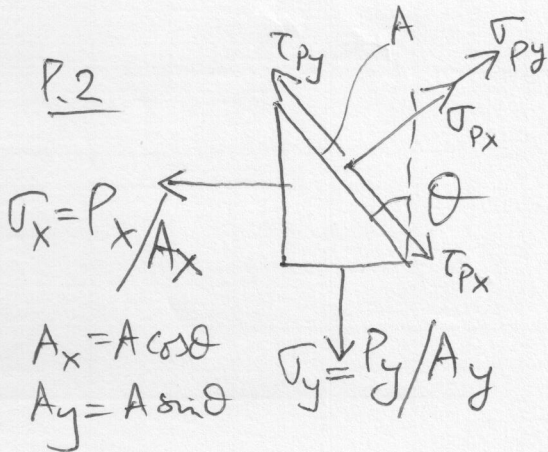
Hookes law:  $P_1 = \sigma_1 A = E_1 \epsilon A$ ;  $P_2 = E_2 \epsilon A \rightarrow \textcircled{3}$ .

$$\Rightarrow P_1 = P_2 \frac{E_1}{E_2}$$

$$\textcircled{1}, \textcircled{3} \rightarrow \left[ P_1 = P \left( \frac{E_1}{E_1 + E_2} \right), P_2 = P \left( \frac{E_2}{E_1 + E_2} \right) \right] \rightarrow \textcircled{4} \rightarrow \text{Ans (b)}$$

$$\textcircled{2}, \textcircled{4} \rightarrow \left[ e = \frac{b}{2} \frac{(E_1 + 3E_2 - 2E_1 - 2E_2)}{(E_1 + E_2)} = \frac{b}{2} \frac{(E_2 - E_1)}{(E_1 + E_2)} \right] \rightarrow \text{Ans (a)}$$

P.2



$$\tau_{px} A = \sigma_x A_x \sin \theta = \sigma_x A \cos \theta \sin \theta$$

$$\tau_{py} A = \sigma_y A_y \cos \theta = \sigma_y A \sin \theta \cos \theta$$

$$\tau = \tau_{px} - \tau_{py} = \frac{(\sigma_x - \sigma_y)}{2} \sin 2\theta$$

$$\tau_{\max} \text{ at } 2\theta = \frac{\pi}{2}, \tau_{\max} = \left( \frac{\sigma_x - \sigma_y}{2} \right)$$

$$60 = \left( \frac{P_x}{500} - \frac{P_y}{2500} \right) \frac{1}{2} = \left( \frac{P_x}{500} - \frac{2P_x}{2500} \right) \frac{1}{2} \Rightarrow P_x = 100000 \text{ N}$$

$P_x = 100 \text{ kN} \leftarrow$   
 $P_y = 200 \text{ kN}.$

$$\Delta \text{ thickness} = \epsilon_z \cdot (10) = -\frac{\nu}{E} (\sigma_x + \sigma_y) (10)$$

$$\text{(Plane stress, } \sigma_z = 0) \rightarrow = \frac{-0.25}{200 \times 10^9 \times 10^{-6}} \left( \frac{10^5}{500} + \frac{2 \times 10^5}{2500} \right) (10)$$

$$= 3.5 \times 10^{-3} \text{ mm}$$