

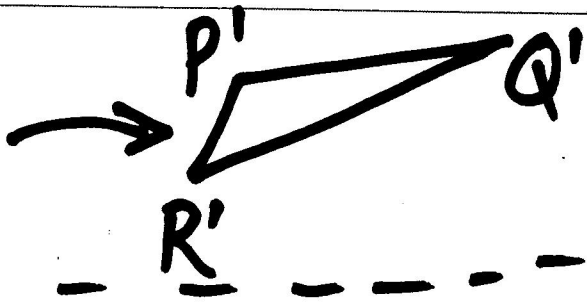
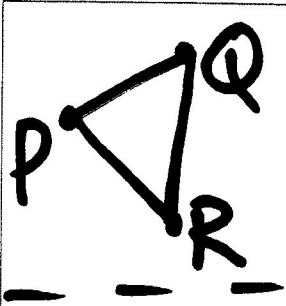
STRAIN ANALYSIS.

(Deformation analysis)



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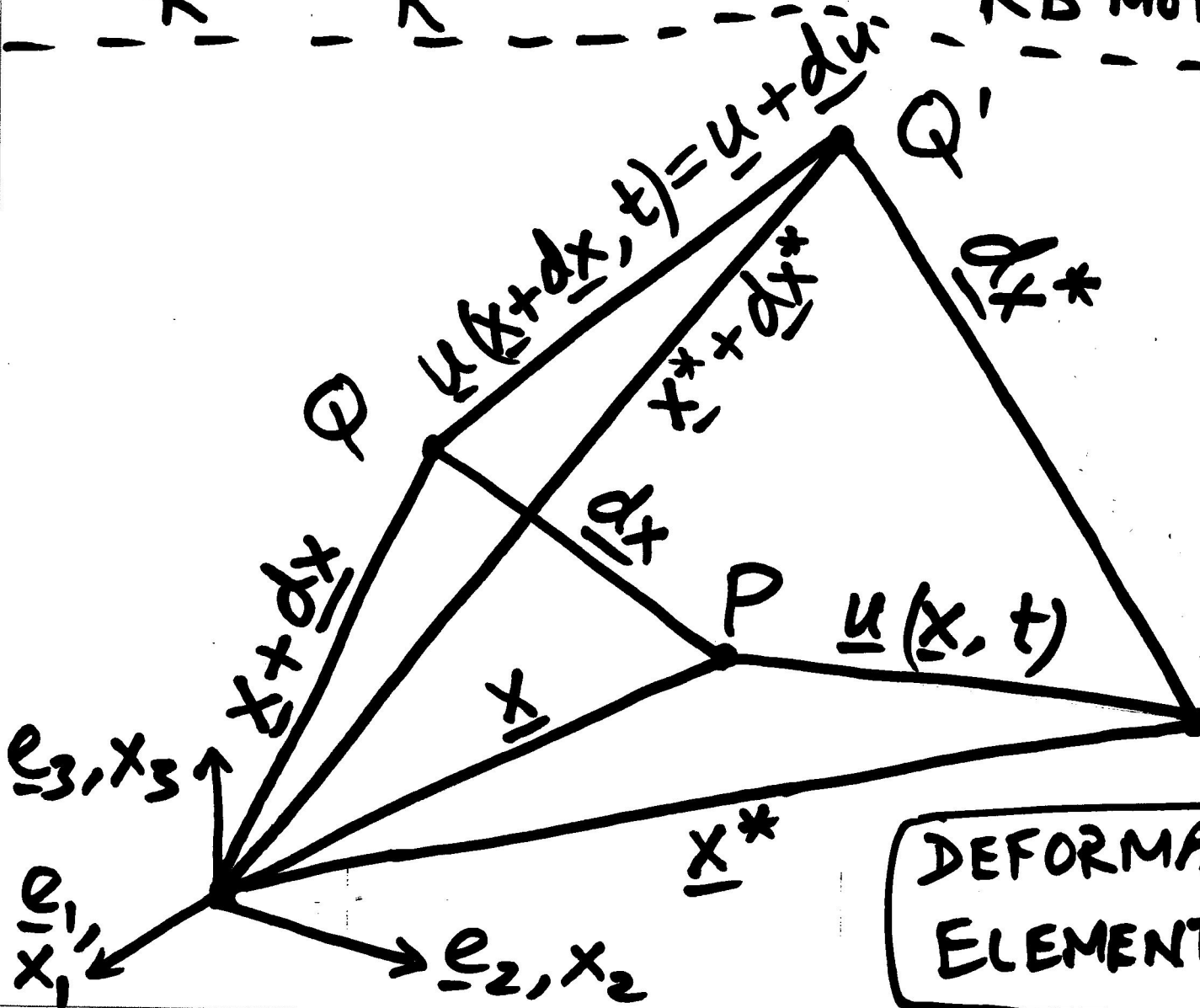
- Kinematics & Nonlinear Strain Tensor.
- Physical interpretation of Strain Tensor Components.
- Change in angle between two arbitrary line elements.
- Strain Transformations.
- Principal Strains & axes.
- Linear Strain Theory.
- Analogies with stress
- Infinitesimal rotation, relative displ, p-strain (Linear).
- Linear Cubical Dilatation.



Total motion =
STRAINING +
RB Motion



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$\underline{x} \equiv \underline{r} = \text{p.v. def}$ (before)
 $= (x_1, x_2, x_3)$
 $= x_i$
 $\underline{u} = \text{displacement}$
 $= (u_1, u_2, u_3)$
 $= u_i$

DEFORMATION OF LINE
ELEMENT PQ AT AN
INSTANT.

X^* = P.V. after deformation

$$\underline{x}^* [x, u, t] = \underline{x} + u [x, t]$$

$$= x_i^* [x_1, x_2, x_3] = x_i + u_i [x_1, x_2, x_3, t]$$

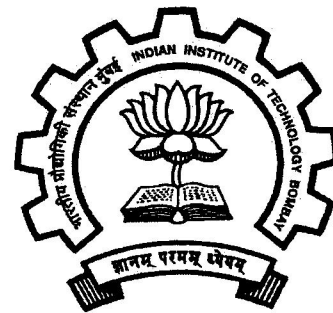
$$\text{STRETCH RATIO} \triangleq (P'Q')^2 - (PQ)^2 = (dx^*)^2 - (dx)^2$$

$$= dx_i^* dx_i^* - dx_i dx_i$$

$$= x_{i,j}^* x_{i,k}^* dx_j dx_k - dx_i dx_i$$

$$= (\delta_{ij} + u_{i,j}) (\delta_{ik} + u_{i,k}) dx_j dx_k - dx_i dx_i$$

$$\text{STRETCH} \triangleq (u_{i,j} + u_{j,i} + u_{k,i} u_{k,j}) dx_i dx_j$$
$$= 2e_{ij} dx_i dx_j \quad \rightarrow 2e_{ij}$$



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where we used simplifications

$$\delta_{ij} \delta_{ik} dx_j dx_k = dx_i dx_i$$

$$\delta_{ij} u_{i,k} dx_j dx_k = u_{j,k} dx_j dx_k$$

$$\delta_{ik} u_{i,j} dx_j dx_k = u_{i,j} dx_j dx_i$$

$$u_{k,j} u_{k,i} dx_j dx_k = u_{k,i} u_{k,j} dx_i dx_j$$

Alternatively can do $dx_i^* = dx_i + du_i$
 $= dx_i + u_{i,j} dx_j$
and get same result.



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→ here ϵ_E is engg extensional strain (physics)

$$M = \text{magnification factor}$$

$$= \frac{1}{2} \frac{(P'Q')^2 - (PQ)^2}{(PQ)^2} = \frac{1}{2} \frac{(dx^*)^2 - (dx)^2}{(dx)^2}$$

$$= e_{ij} n_i n_j$$

where $\frac{dx_i}{dx} = n_{ij} \frac{dx_j}{dx} = n_j$

$$= \frac{1}{2} \left[\left(\frac{dx^*}{dx} \right)^2 - 1 \right]$$

unit normal along PQ

$$= \frac{1}{2} \left[(1 + \epsilon_E)^2 - 1 \right]$$

Summary

$$= \epsilon_E + \frac{\epsilon_E^2}{2}$$

$$M = \frac{1}{2} \frac{(dx^*)^2 - (dx)^2}{(dx)^2} = e_{ij} n_i n_j$$

where,

→ STRAIN-DISPLACEMENT RELATION.

$$= \epsilon_E + \frac{\epsilon_E^2}{2}$$

$$\epsilon_E = \frac{dx^* - dx}{dx}, \quad e_{ij} = \frac{1}{2} [u_{i,j} + u_{j,i} + u_{k,i} u_{k,j}]$$

(i) $E_{ij} \rightarrow$ strain tensor.
 \rightarrow measure of straining

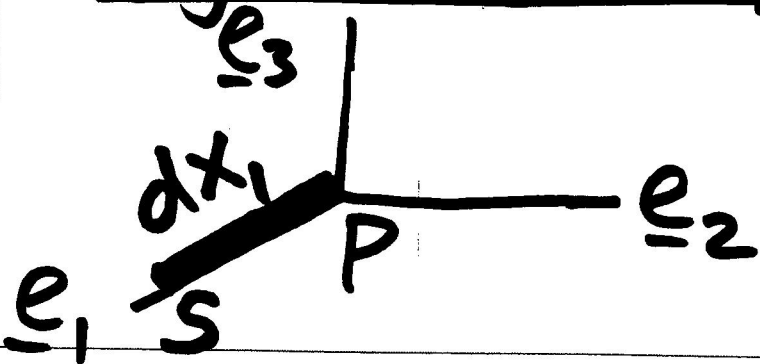
$$dx^* \neq dx \Rightarrow E_{ij} \neq 0$$

$= E_{ji}$ (symmetric by definition).

(ii) R.B motion does not affect M

(iii) M analogous to N (normal stress).
(compare with $N = \sigma_{ij} n_i n_j$)

Physical interpretation of E_{ij} (nonlinear).



Normal strains (diagonal comp's)
Consider element PS
along x_1 axis.



$$\begin{aligned} \epsilon_E &= \frac{dx^* - dx}{dx} = \frac{(dx^*)^2 - (dx)^2}{(dx)^2 (\epsilon_E + 2)} \\ &= \frac{2e_{ij} n_i n_j}{(\epsilon_E + 2)} = \frac{2e_{11}}{(\epsilon_{E1} + 2)} \end{aligned}$$



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$$\Rightarrow \boxed{\epsilon_{E1} = \sqrt{1 + 2e_{11}} - 1} \rightarrow \textcircled{2}$$

$$M_1 = \epsilon_{E1} + \frac{\epsilon_{E1}^2}{2} = e_{11}$$

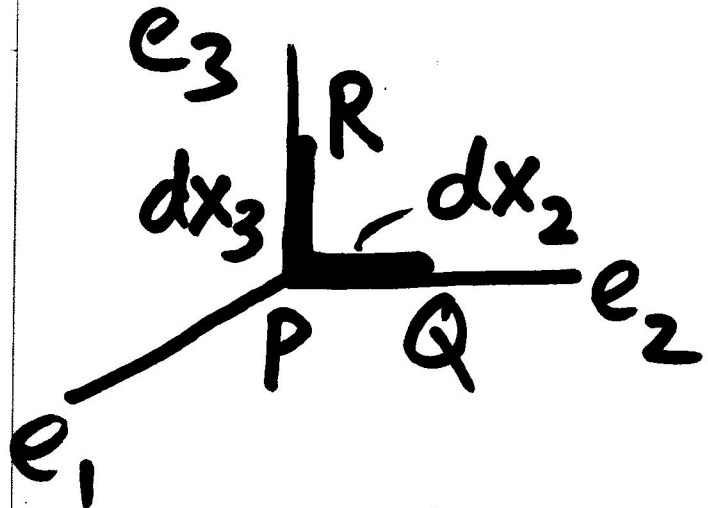
Similarly for e_{22}, ϵ_{E2}
 e_{33}, ϵ_{E3}

← used
 $\underline{n} = (1, 0, 0)$
 i.e. $n_1 = 1, n_2 = n_3 = 0$
 $\epsilon_E \rightarrow \epsilon_{E1}$
 ∴ element in
 1-dir.

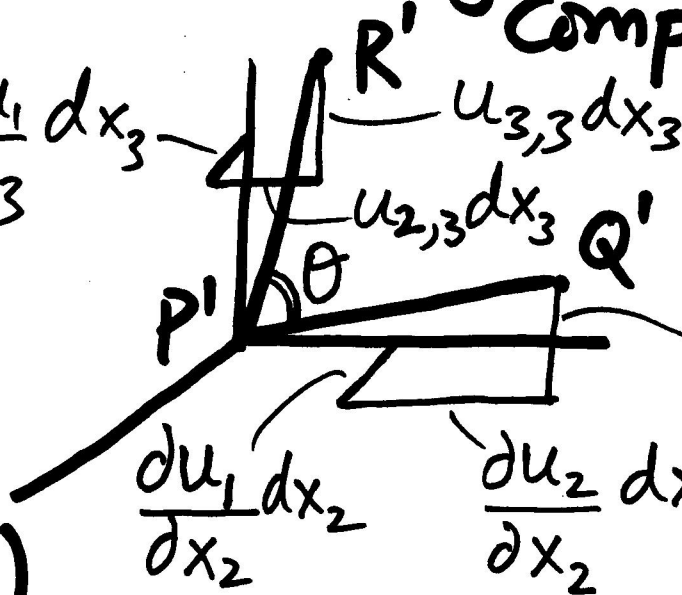
Shear strains (off-diagonal comp's)



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$$\frac{\partial u_1}{\partial x_3} dx_3$$



$$\underline{u} = \underline{u}_P = (u_1, u_2, u_3)$$

$$\begin{aligned} \underline{P'Q'} \cdot \underline{P'R'} &= (P'Q')(P'R') \cos \theta = (\epsilon_{E2} + 1)_P dx_2 (\epsilon_{E3} + 1)_P dx_3 \\ &= \sqrt{1 + 2\epsilon_{22}} \sqrt{1 + 2\epsilon_{33}}_P dx_2 dx_3 \cos \theta \xrightarrow{\cos \theta} (a) \end{aligned}$$

ALSO

$$\begin{aligned} \underline{P'Q'} \cdot \underline{P'R'} &= \underline{dx^*}_{PQ} \cdot \underline{dx^*}_{PR} = dx_i^* / PQ \quad dx_i^* / PR \\ &= \left(\frac{\partial x_i^*}{\partial x_j} \right)_P dx_j / PQ \left(\frac{\partial x_i^*}{\partial x_R} \right)_P dx_R / PR \end{aligned}$$

use $dx_j|_{PQ} = (0, dx_2, 0)$, $dx_k|_{PR} = (0, 0, dx_3)$

$$\underline{P'Q'} \cdot \underline{P'R'} = \left(\frac{\partial x_i^*}{\partial x_2} \quad \frac{\partial x_i^*}{\partial x_3} \right)_P dx_2 dx_3$$



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Recall, p. 3,

$$x_{i,j}^* x_{i,k}^* dx_j dx_k - dx_j dx_k = 2e_{jk} dx_j dx_k$$

$= dx_k \delta_{jk}$

$$(x_{i,j}^* x_{i,k}^* - \delta_{jk}) dx_j dx_k = 2e_{jk} dx_j dx_k$$

$$\underline{P'Q'} \cdot \underline{P'R'} = (x_{i,2}^* x_{i,3}^*)_P dx_2 dx_3 = 2(e_{23})_P dx_2 dx_3 \rightarrow (6)$$

(a), (b) \Rightarrow

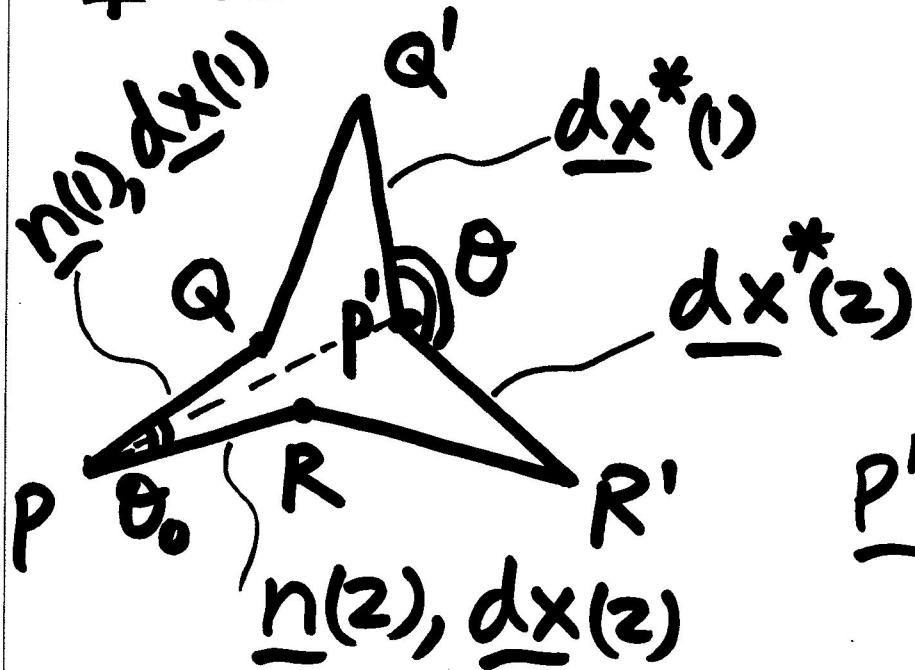
Similarly for e_{13}, e_{12} \leftarrow

$$2e_{23} = \sqrt{1+2e_{22}} \sqrt{1+2e_{33}} \cos\theta = (1+E_{E2})(1+E_{E3}) \cos\theta \rightarrow (3)$$

Angle between two line elements after deformation — Generalization of shear strain interpretation.



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Consider two line elements
 (1) \rightarrow PQ, (2) \rightarrow PR before
 & after deformation.

$$\underline{P'Q'} \cdot \underline{P'R'} = \underline{dx}^*(1) \cdot \underline{dx}^*(2)$$

$$= dx_i^*(1) dx_i^*(2)$$

$$= (dx_i + u_{i,j} dx_j)_{(1)} (dx_i + u_{i,k} dx_k)_{(2)}$$

$$= dx_i(1) dx_i(2) + dx_i(1) dx_j(2) [u_{i,j} + u_{j,i} + u_{k,i} u_{k,j}]_{(2)}$$

\downarrow (A) $\quad \underline{P}$

$$\text{Also } \underline{P'Q'} \cdot \underline{P'R'} = \sqrt{(dx^*(1))^2} \sqrt{(dx^*(2))^2} \cos \theta$$



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see ①

P.4

$$= \sqrt{dx_i(1) dx_i(1) + 2e_{ij} dx_i(1) dx_j(1)} \sqrt{dx_p(2) dx_p(2) + 2e_{pq} dx_p(2) dx_q(2)} \cos \theta \rightarrow (B)$$

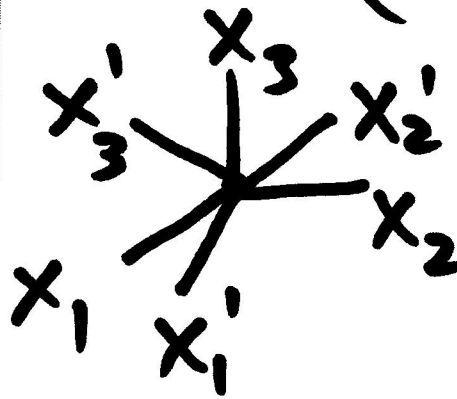
Divide (A) & (B) by $dx(1) dx(2)$, note that $n_i^{(1)} = \frac{dx_i(1)}{dx(1)}$
 $n_j(2) = \frac{dx_j(2)}{dx(2)}$, $n_i(1) n_i(1) = n_j(2) n_j(2) = 1$,
 $n_i(1) n_i(2) = \cos \theta_0$, you get

all strain comps at P

$$\cos \theta = \frac{\cos \theta_0 + 2 n_i(1) n_j(2) e_{ij}}{\sqrt{1 + 2 n_r(1) n_s(1) e_{rs}} \sqrt{1 + 2 n_p(2) n_q(2) e_{pq}}} \rightarrow (4)$$

Strain Transformations.

(ie, transf of coords.)



① p.4

Consider element along x'_1 .

$$M = e_{ij} n_i n_j = e_{ij} a_{1i} a_{1j} \rightarrow n_i = a_{1i}$$

$$\text{Also } M = e'_{ij} n'_i n'_j = e'_{11}$$

$$\therefore \underline{n} = (a_{11}, a_{12}, a_{13})$$

$$\therefore \underline{n}' = (1, 0, 0)$$

$$\Rightarrow e'_{11} = e_{ij} a_{1i} a_{1j} \rightarrow (i)$$

③ p.9 gives θ between elements along x'_2, x'_3 axes after deformation as $\cos \theta = \frac{2e'_{23}}{\sqrt{1+2e'_{22}} \sqrt{1+2e'_{33}}} \rightarrow (ii)$



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This is in terms of strains referred to x'_i system. We can get $\cos\theta$ in terms of strains referred to x_i system by using (4) p. 11. Here, $\theta_0 = 90^\circ$, line (1) is x'_2 axis so $n_i(1) = a_{2i} = (a_{21}, a_{22}, a_{23})$, line (2) is x'_3 axis, so $n_j(2) = a_{3j} = (a_{31}, a_{32}, a_{33})$.

$$\Rightarrow \cos\theta = \underbrace{\cancel{\cos 90} + 2a_{2i}a_{3j}e_{ij}}_{\sqrt{1 + 2a_{2r}a_{2s}e_{rs}}} \underbrace{\phantom{\cancel{\cos 90} + 2a_{2i}a_{3j}e_{ij}}}_{\sqrt{1 + 2a_{3p}a_{3q}e_{pq}}} \rightarrow (iii)$$

see (i) p. 12 \rightarrow e'_{22} e'_{33}

Compare (ii), (iii), p. 12, 13,

$$e'_{\substack{12 \\ 13}} = a_{\substack{2i \\ 1}} a_{\substack{3j \\ 3}} e_{ij} \rightarrow (iv)$$

Combine transf. law for normal & shear strains (ie (i), (iv), p 12, 14)

$$e'_{ij} = a_{ir} a_{js} e_{rs}$$

→ ⑤

STRAIN TRANSF LAW

Same as STRESS TRANSF LAW.

So e_{ij} is 2nd order tensor.

So e_{ij} analogous (mathematically) to σ_{ij}
Both are symmetric.



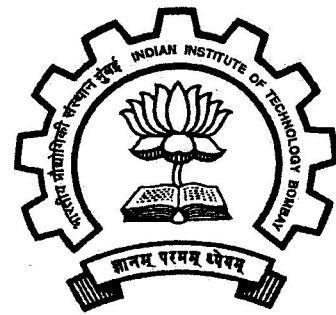
Principal Strains & Directions.

Eigenvalue problem (due to σ_{ij} e_{ij} analogy) is

$$\textcircled{6} \leftarrow (e_{ij} - \lambda \delta_{ij}) n_j = 0 = \left(\underline{\underline{e}} - \lambda \underline{\underline{I}} \right) \underline{\underline{n}}$$

This gives p -strains $\lambda = M(1), M(2), M(3)$ and p -directions $n(1), n(2), n(3)$, ie a coord system x'_1, x'_2, x'_3 such that e_{ij} gets diagonalized, ie $e'_{ij} = 0$ for $i \neq j$.

Analogies: $M(N)$ stationary at p -axes of strain (stress). Also p -axes orthogonal



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$$\text{From } \textcircled{1} \text{ p. 4, } M = \epsilon_E + \frac{\epsilon_E^2}{2}$$

$$\Rightarrow dM = d\epsilon_E(1 + \epsilon_E)$$

Now $\epsilon_E =$ physical quantity, i.e. engg extensional strain

$$= \frac{dx^* - dx}{dx} = \frac{dx^*}{dx} - 1$$

so $(1 + \epsilon_E) > 0 \because dx^*, dx$ are lengths (+ve).

So, M stationary $\Rightarrow dM = 0 = d\epsilon_E \Rightarrow \epsilon_E$ stationary

Thus $M(1), M(2), M(3)$ stationary values of magnification factor correspond to $\epsilon'_E(1), \epsilon'_E(2), \epsilon'_E(3)$ i.e. stationary values of engg ext str^{which act} along x'_1, x'_2, x'_3 i.e. $\eta(1), \eta(2), \eta(3)$ axes

Thus CE (Characteristic Eqn) is

$$\lambda^3 - J_1 \lambda^2 + J_2 \lambda - J_3 = 0$$

$$J_1 = e_{ii}, \quad J_2 = e_{11}e_{22} + e_{22}e_{33} + e_{33}e_{11} \\ - e_{12}^2 - e_{23}^2 - e_{13}^2$$

$$J_3 = \det \underline{\underline{e}}$$

$J_1, J_2, J_3 \rightarrow$ strain invariants (analogous to I_1, I_2, I_3)

$\lambda(i) = M(i), \text{ etc, (analogous to } N(i), \text{ etc).}$



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Linear Theory - Small displacement gradient theory.



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$$u_{i,j} \ll 1$$

$$\Rightarrow e_{ij} = \frac{1}{2}(u_{i,j} + u_{j,i})$$

$$\Rightarrow du_i = u_{i,j} dx_j = \text{small} \Rightarrow dx^* \approx dx$$

$$\Rightarrow M = \frac{1}{2} \frac{(dx^*)^2 - (dx)^2}{(dx)^2} \approx \frac{(dx^* - dx)2dx}{2(dx)^2}$$

$$= \epsilon_E \quad (\text{can also get directly from } \textcircled{1} \text{ P.4, putting } \epsilon_E \ll 1).$$

$$= e_{ij} n_i n_j$$

1A

Physical interpretation of e_{ij} (Linear)



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$$u_{i,j} \ll 1 \Rightarrow e_{ij} \ll 1$$

$$\textcircled{2} \text{ p.7} \Rightarrow \epsilon_{E1} = e_{11}$$

(can also get from $\epsilon_E = e_{ij} n_i n_j$ valid for linear theory with $\underline{n} = \{1, 0, 0\}$)

$$\textcircled{3} \text{ p.9} \Rightarrow e_{23} = \frac{1}{2} \cos \theta = \frac{\sin(\frac{\pi}{2} - \theta)}{2} = \frac{\alpha}{2}$$

α = change in angle between PQ & PR originally along x_2, x_3 axes, resp. α = small $\therefore u_{i,j} \ll 1$ i.e. $e_{ij} \ll 1$

e_{23} = tensorial shear strain

$\textcircled{-19-}$

Note: $e_{23} = \frac{\alpha}{2} = \frac{\gamma_{23}}{2}$
 γ_{23} = engg shear strain.

Note:

$$\gamma_{ij} = 2\epsilon_{ij}, \quad i \neq j$$

$\epsilon_{ij} \rightarrow$ Tensorial strains. They transform as a tensor for all i, j .

$\gamma_{ij} \rightarrow$ Engg shear strains

$\gamma_{ij} = \epsilon_{ij}, \quad i \neq j \rightarrow$ one transf law.

$\gamma_{ij} = 2\epsilon_{ij}, \quad i \neq j \rightarrow$ another transf law.



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Analogies with $\underline{\underline{\sigma}}$

(i) $\hat{e}_{ij} = e_{ij} - \frac{1}{3} \delta_{ij} e_{mm} \rightarrow \textcircled{7}$
deviatoric = strain - spherical.

(ii) Pure shear state of strain - iff $e_{mm} = 0$
Physically it means that ^{at point P} there exist
three mutually perpendicular directions
 $\underline{n}^{(1)}, \underline{n}^{(2)}, \underline{n}^{(3)}$ along which no engineering
extensional strain occurs, ie $e_{E1}, e_{E2},$
 e_{E3} ^{resply} are zero.



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Infinitesimal Rotation, Relative

Displacement, P-strains — LINEAR



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$$du_i = u_{i,j} dx_j = \frac{1}{2} \left[\underbrace{(u_{i,j} + u_{j,i})}_{e_{ij} \text{ for Linear}} + \underbrace{(u_{i,j} - u_{j,i})}_{-w_{ji} = w_{ij}} \right] dx_j$$
$$= [e_{ij} + w_{ij}] dx_j$$

RB motion only $\Rightarrow e_{ij} = 0 \Rightarrow du_i = w_{ij} dx_j$

Linear Rot Tensor.

$$du_i = \begin{bmatrix} 0 & w_{12} & w_{13} \\ -w_{12} & 0 & w_{23} \\ -w_{13} & -w_{23} & 0 \end{bmatrix} \begin{Bmatrix} dx_1 \\ dx_2 \\ dx_3 \end{Bmatrix} = \underline{\underline{\omega}} \times \underline{\underline{dx}}$$

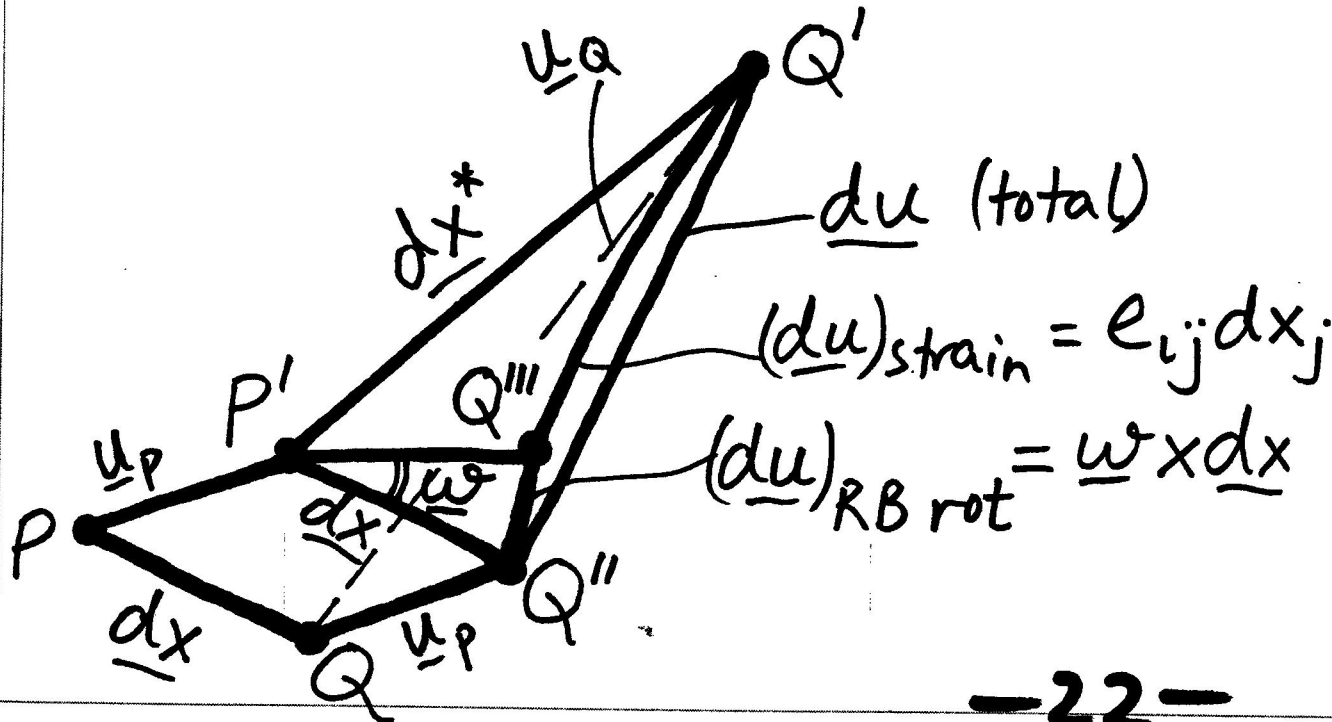
Linear Rot. vector

$$\underline{\underline{\omega}} = \{w_{32}, w_{13}, w_{21}\}^T$$

ie, due to skew symmetry of $\underline{\underline{\omega}} \equiv \omega_{ij}$ it can be written as $\underline{\underline{\omega}} \equiv \omega_i$ \therefore only three independent components.
 So $\underline{\underline{\omega}} \times \underline{dx} = \text{Rot. Comp. of. } \underline{du}$



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so relative motion $\underline{du} = \text{RB motion} + \text{Straining.}$

Another explanation of P-strains:

Consider straining only.

(LINEAR)

$$e_i \frac{du_i}{dx} = \frac{e_{ij} dx_j}{dx} = e_{ij} n_j \rightarrow \textcircled{8}$$

= relative displ per unit length of element due to straining only.

In general e_i neither along PQ nor $P'Q'$, it is along $Q''Q'$. (element)
We seek orientation of PQ , i.e. n_i , such that ^{the} element remains perpendicular (after deformation) to the plane of particles that it



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was originally (before def.) perpendicular to. Since RB motion is excluded, this means that e_i and n_i are in same direction,

$$e_i = \gamma n_i \Rightarrow e_{ij} n_j = \gamma \delta_{ij} n_j$$
$$(e_{ij} - \gamma \delta_{ij}) n_j = 0$$

Note that this does not mean that direction PQ remains same after deformation, since RB motion will be present in general. All it means is that dir. PQ remains



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Same when RB motion filtered out. So with RB motion included we would have

$P'Q'''$ and $Q'''Q'$ and $P'Q'$ having same direction if PQ is a principal element (ie directed along p-axis).

Note that e_i & t_i are analogous.

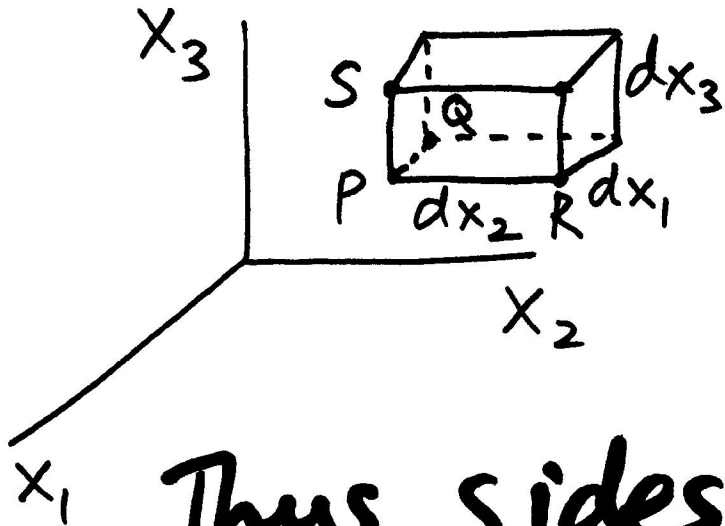


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Linear Cubical Dilatation (Volumetric Strain).



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Consider rectangular parallelepiped with sides oriented along p-coord axes x_1, x_2, x_3 .

Thus sides remain perpendicular after deformation since they lie along p-axes.

$$dV^* = dx_1^* dx_2^* dx_3^* = (1 + \epsilon_{E1})(1 + \epsilon_{E2})(1 + \epsilon_{E3})$$

$$D \triangleq \frac{dV^* - dV}{dV} = \epsilon_{E1} + \epsilon_{E2} + \epsilon_{E3} \quad * dx_1 dx_2 dx_3$$



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$$D = \epsilon_{E1} + \epsilon_{E2} + \epsilon_{E3} \rightarrow \text{Engg Strains}$$

$$= \epsilon_{11} + \epsilon_{22} + \epsilon_{33} \rightarrow \text{linear normal strains}$$

$$= \gamma(1) + \gamma(2) + \gamma(3) \rightarrow p\text{-strains}$$

9

ϵ_{kk}

Here we have linearized by neglecting products of engg strains.

NOTE: \hat{D} = cubical dilatation for Deviatoric strain tensor = 0 $\therefore \hat{\epsilon}_{ii} = 0$

So Deviatoric part of strain tensor causes no volumetric strain, it causes only distortion strain (shear).
Similarly spherical part causes only vol. strain and no distortion.