

Lecture 2

STRESS ANALYSIS



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L2

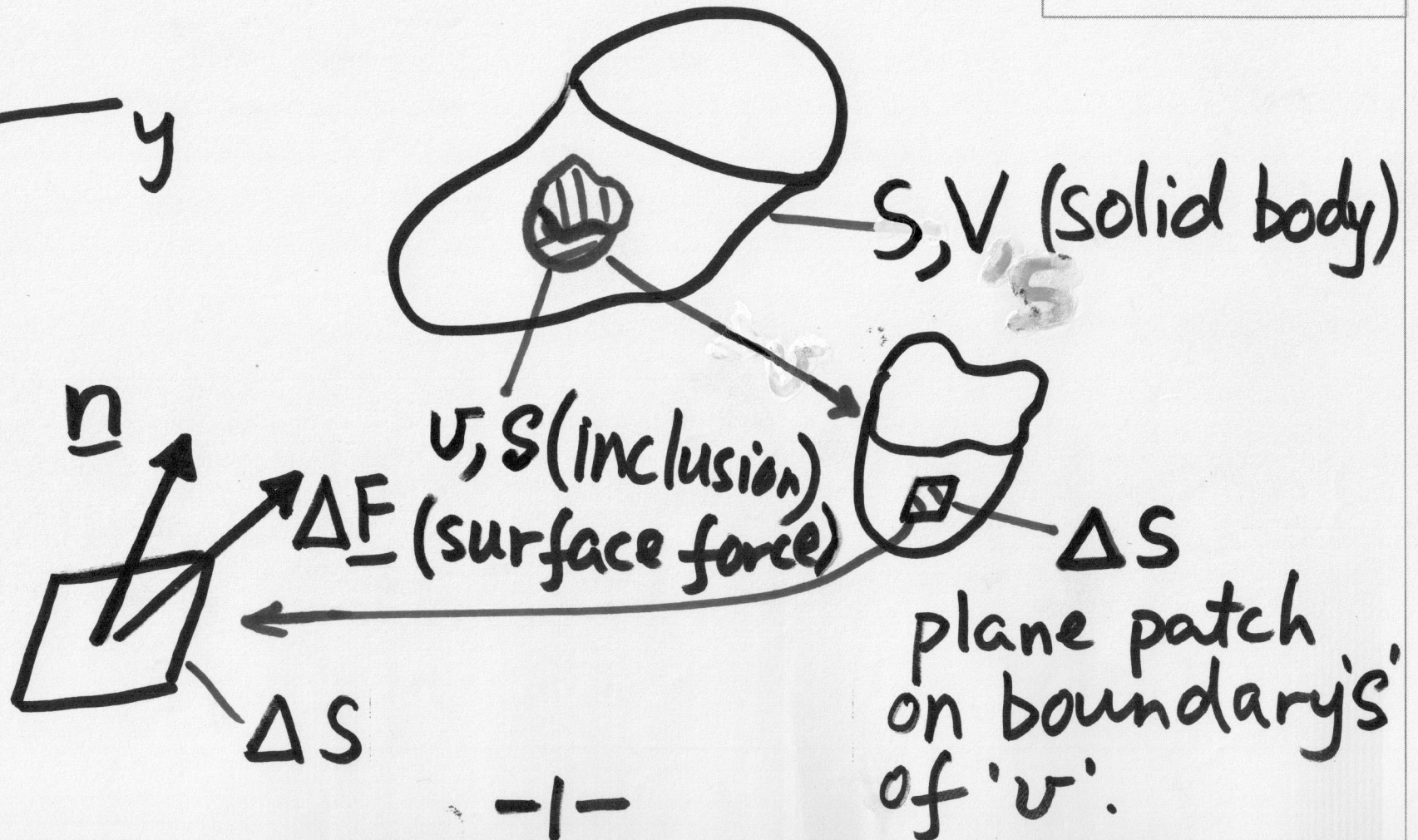
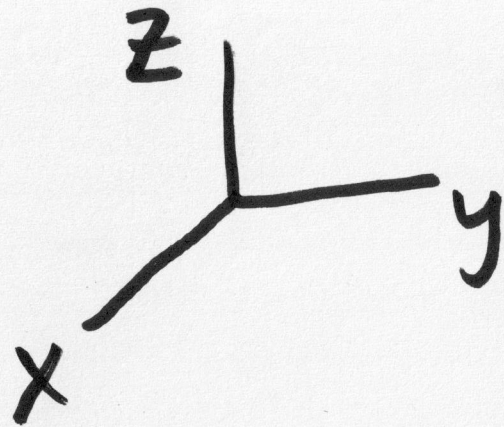
- Surface force, stress/
traction vector
- stress tensor
- Body forces & body moments
- Relating stress vector with stress tensor
- Symmetry of stress tensor
- Normal & shearing stresses.



Surface force, Stress/Traction vector:



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$$\underline{t} = \lim_{\Delta S \rightarrow 0} \frac{\Delta \underline{F}}{\Delta S}$$



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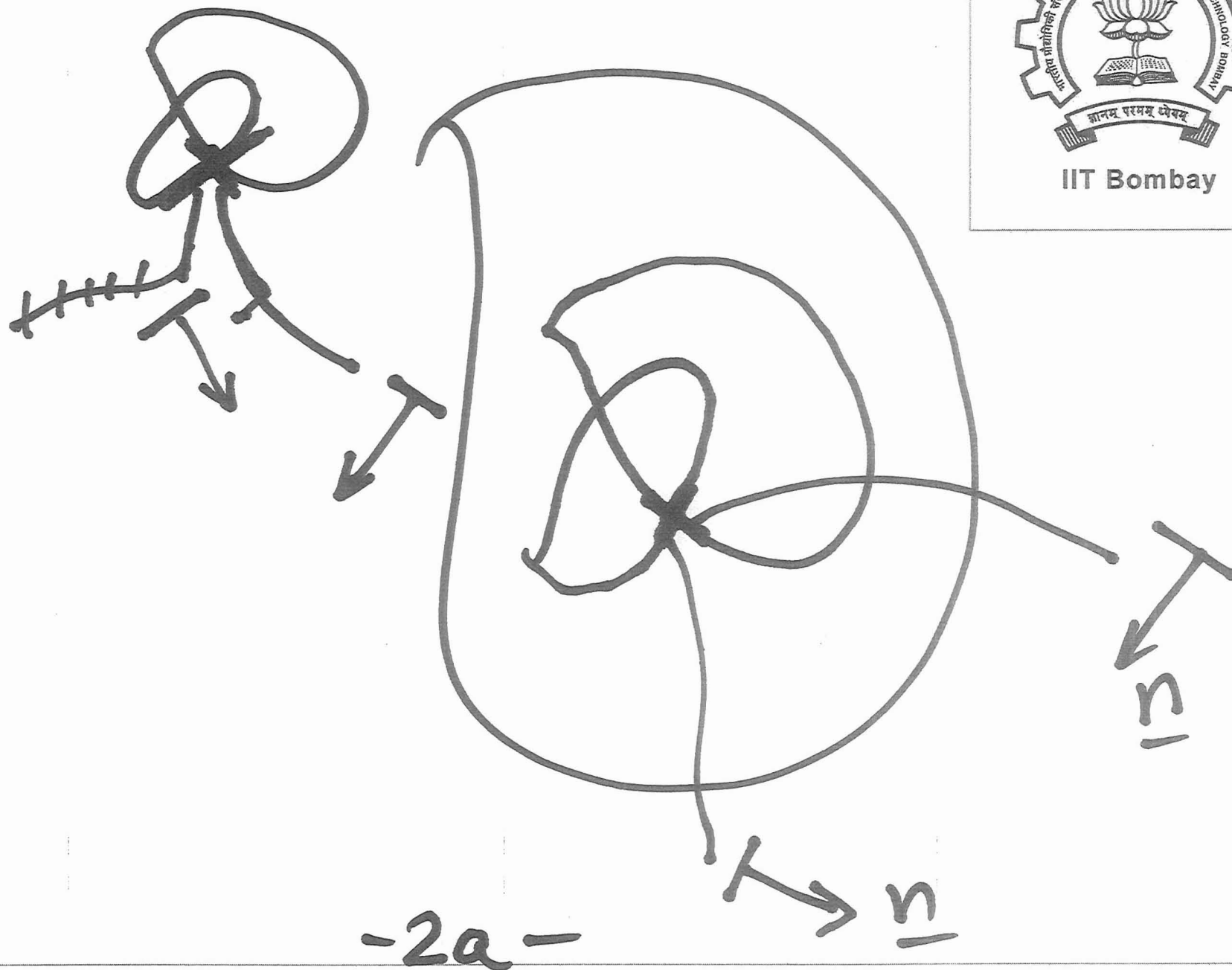
↳ definition of stress/traction vector
Stress vector depends on:

- 1) Location of ΔS , i.e., $P(x, y, z)$
- 2) Orientation of ΔS , i.e., \underline{n}



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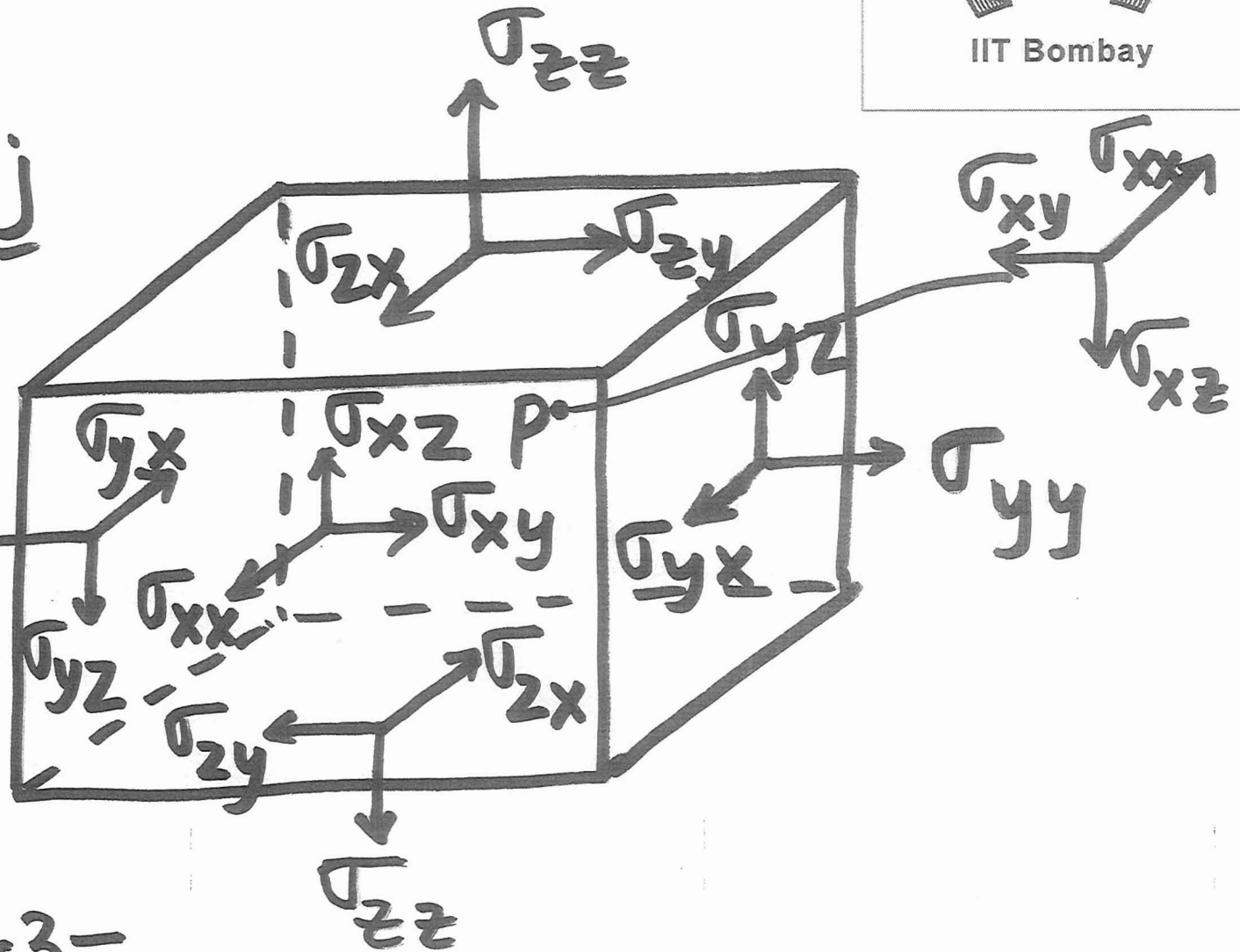
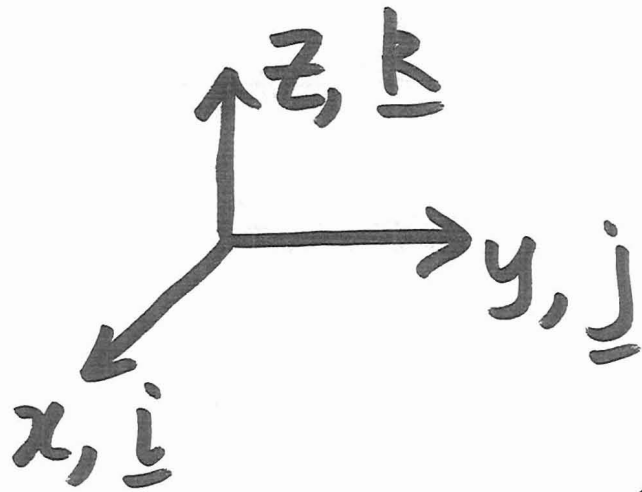


Stress Tensor :



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Infinitesimal
parallelepiped
at point $P(x, y, z)$



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$$\left. \begin{aligned} \underline{t}_{(j)} &= \sigma_{yx} \underline{i} + \sigma_{yy} \underline{j} + \sigma_{yz} \underline{k} \\ \underline{t}_{(k)} &= \sigma_{zx} \underline{i} + \sigma_{zy} \underline{j} + \sigma_{zz} \underline{k} \\ \underline{t}_{(i)} &= \sigma_{xx} \underline{i} + \sigma_{xy} \underline{j} + \sigma_{xz} \underline{k} \end{aligned} \right\} \rightarrow \star$$

$$\begin{Bmatrix} \underline{t}_{(i)} \\ \underline{t}_{(j)} \\ \underline{t}_{(k)} \end{Bmatrix} = \begin{bmatrix} \sigma_{xx} & \sigma_{xy} & \sigma_{xz} \\ \sigma_{yx} & \sigma_{yy} & \sigma_{yz} \\ \sigma_{zx} & \sigma_{zy} & \sigma_{zz} \end{bmatrix} \begin{Bmatrix} \underline{i} \\ \underline{j} \\ \underline{k} \end{Bmatrix} = \underline{\underline{\sigma}} \begin{Bmatrix} \underline{i} \\ \underline{j} \\ \underline{k} \end{Bmatrix}$$

→ Stress Tensor $\underline{\underline{\sigma}}$ at $P(x, y, z)$

σ depends on:

1) Coordinates (x, y, z)
of P

2) Loads, displacements applied
on body.

σ does not depend on \underline{n} , i.e.
orientation of plane at P.



Body forces, Body moments



- Surface force $\Delta \underline{F}$ due to direct contact across ΔS
- Body forces, body moments due to action at a distance (eg. gravitational, magnetic field).
- B_f, B_m , proportional to mass/vol.

$$\underline{f}_v = \lim_{\Delta m \rightarrow 0} \frac{\Delta \underline{F}_v}{\Delta m} = \lim_{\Delta V \rightarrow 0} \frac{1}{\rho} \frac{\Delta \underline{F}_v}{\Delta V}$$

$$= \frac{1}{\rho} \tilde{\underline{f}}_v$$

$$\underline{m}_v = \lim_{\Delta m \rightarrow 0} \frac{\Delta \underline{M}_v}{\Delta m} = \lim_{\Delta V \rightarrow 0} \frac{1}{\rho} \frac{\Delta \underline{M}_v}{\Delta V} = \frac{1}{\rho} \tilde{\underline{m}}_v$$

$\rho(x, y, z) \rightarrow$ density

$\underline{f}_v(x, y, z), \tilde{\underline{f}}_v(x, y, z) \rightarrow$ BF per unit mass, vol

$\underline{m}_v(x, y, z), \tilde{\underline{m}}_v(x, y, z) \rightarrow$ BM " " " "



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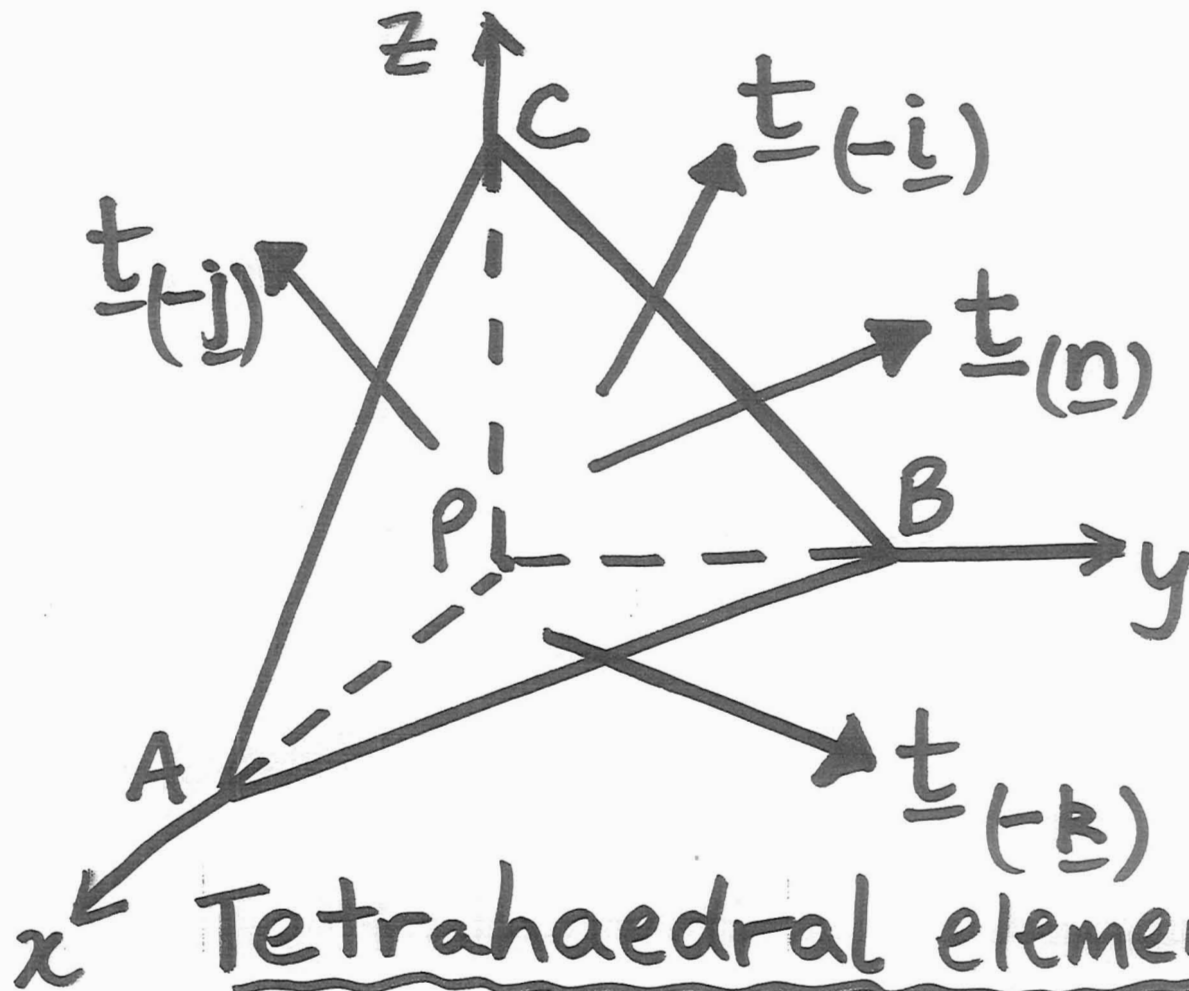
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Relating \underline{t} with $\underline{\sigma}$, \underline{n} at P

— Cauchy relation:



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• $-\underline{i}$, $-\underline{j}$, $-\underline{k}$, \underline{n} , are normals to planes PBC, PAC, PAB, ABC, resply.

Tetrahaedral element at P.



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$$\cdot \underline{t}_{(-i)}, \underline{t}_{(-j)}, \underline{t}_{(-k)}, \underline{t}_{(n)}$$

are AVERAGE stress vectors
on planes PBC, PAB, PAC, ABC, resply.

Equilibrium of forces \Rightarrow

$$\begin{aligned} & \underline{t}_{(-i)} \Delta PBC + \underline{t}_{(-j)} \Delta PAC \\ & + \underline{t}_{(-k)} \Delta PAB + \underline{t}_{(n)} \Delta ABC \\ & + \underline{f}_b * \left(\frac{1}{3} h \Delta ABC \right) = \underline{0} \end{aligned}$$

vol of tetra



÷ by ΔABC

Note $\frac{\Delta PBC}{\Delta ABC} = n_x$,

$$\frac{\Delta PAC}{\Delta ABC} = n_y , \quad \frac{\Delta PAB}{\Delta ABC} = n_z$$

where,

$$\underline{n} = n_x \underline{i} + n_y \underline{j} + n_z \underline{k}$$

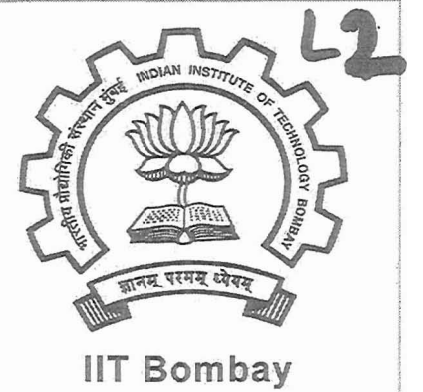
$$\Rightarrow \underline{t}(\underline{i}) n_x + \underline{t}(-\underline{j}) n_y + \underline{t}(-\underline{k}) n_z$$

$$+ \underline{t}(\underline{n}) + \underline{f}_L \cdot \frac{1}{3} h = 0$$

Let tetra shrink to P
such that \underline{n} unchanged.

$\Rightarrow h \rightarrow 0$, all \underline{t} 's are exact
(not AVERAGE) at P .

Use $\underline{t}_{(-i)}$, $\underline{t}_{(-j)}$, $\underline{t}_{(-k)}$ in terms of
components of $\underline{\underline{\sigma}}$ (see p.4, ★)





$$\Rightarrow \underline{t}_{(n)} = (\sigma_{xx} n_x + \sigma_{yx} n_y + \sigma_{zx} n_z) \underline{i} \\ + (\sigma_{xy} n_x + \sigma_{yy} n_y + \sigma_{zy} n_z) \underline{j} \\ + (\sigma_{xz} n_x + \sigma_{yz} n_y + \sigma_{zz} n_z) \underline{k}$$

→ Cauchy relation, \underline{t} in terms of $\underline{\sigma}$, \underline{n} , at P.

let $\underline{i} \rightarrow \underline{e}_1$, $\underline{j} \rightarrow \underline{e}_2$, $\underline{k} \rightarrow \underline{e}_3$, $x \rightarrow 1$, $y \rightarrow 2$, $z \rightarrow 3$

$$\underline{t}_{(n)} = \sum \sum \sigma_{ij} n_i \underline{e}_j \rightarrow \textcircled{1}$$

Cauchy relation



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$$\underline{t} = \sum_{j=1}^3 \sum_{i=1}^3 \sigma_{ij} n_i \underline{e}_j$$

①

or

$$\underline{t} = \underline{\underline{\sigma}} \underline{n} \quad \text{ie } \{t\} = [\sigma] \{n\}$$

where $\{t\} = \{t_1 \ t_2 \ t_3\}^T = \{t_x \ t_y \ t_z\}^T$

$[\sigma] = \text{stress tensor (p.4)}$

$\{n\} = \{n_1 \ n_2 \ n_3\}^T = \{n_x \ n_y \ n_z\}^T$

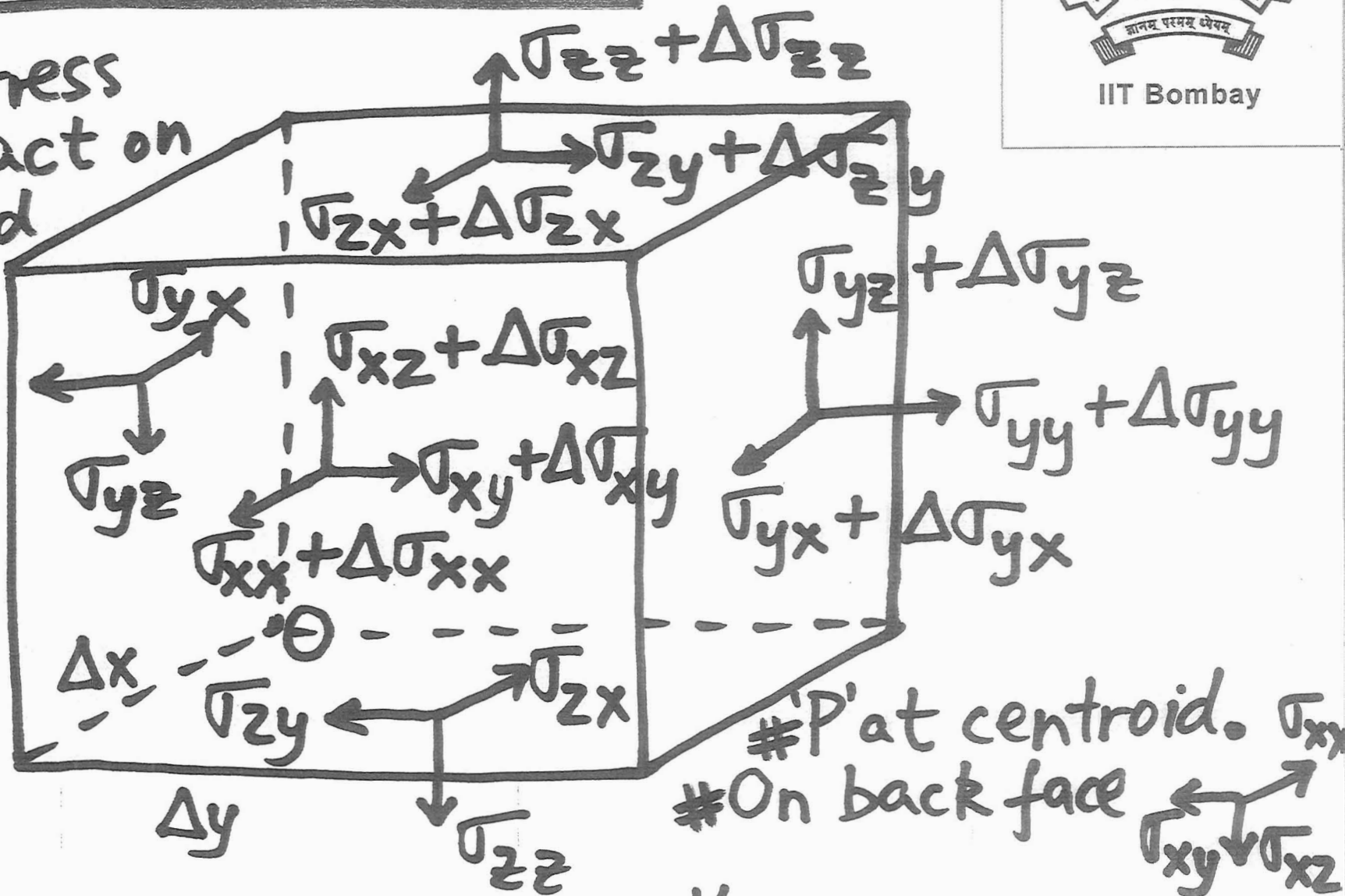
Symmetry of $\underline{\underline{\sigma}}$



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All stress
comps act on
centroid
of face.

σ_{yy}
 Δz



Moment equilibrium:

$$\Sigma \underline{M} = \underline{0} \text{ about } P.$$

$$= M_x \underline{i} + M_y \underline{j} + M_z \underline{k}$$

M_x, M_y, M_z are moments about x, y, z , axes, resply, thru P .

$$M_x = \left(\sigma_{yz} + \frac{\partial \sigma_{yz}}{\partial y} \Delta y + \sigma_{yz} \right) \Delta x \Delta z \Delta y / 2$$

$$- \left(\sigma_{zy} + \frac{\partial \sigma_{zy}}{\partial z} \Delta z + \sigma_{zy} \right) \Delta x \Delta y \Delta z / 2$$

$$+ \left(\tilde{m}_{bx} + \tilde{f}_{by} \alpha \Delta z + \tilde{f}_{bz} \beta \Delta y \right) \Delta x \Delta y \Delta z = 0$$



$$\# \div \Delta x \Delta y \Delta z$$

Neglect higher order terms

↓ $(\sigma_{yz,y} \Delta y; \sigma_{zy,z} \Delta z; \bar{f}_{by} \propto \Delta z;$
(eqvt to shrink to P) $\bar{f}_{bz} \propto \Delta y)$

Assume no body moments, ie $\bar{m}_{bx} = 0$

\Rightarrow

$$\sigma_{yz} = \sigma_{zy}$$

Similarly, $\sigma_{xy} = \sigma_{yx} \rightarrow$ from $M_z = 0$

$$\sigma_{xz} = \sigma_{zx} \rightarrow$$
 from $M_y = 0$



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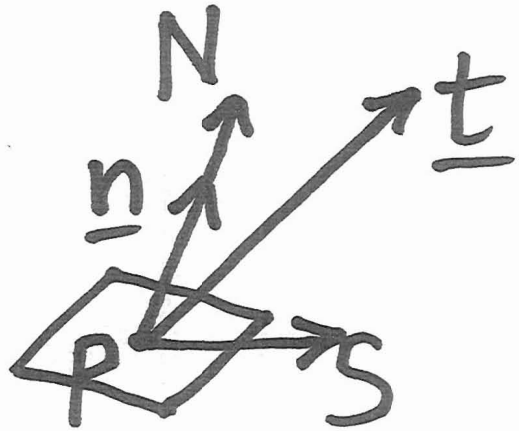
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Normal (N) and Shear (S)

Stresses on plane (n)



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$$\underline{n} = n_x \underline{i} + n_y \underline{j} + n_z \underline{k}$$

$$\underline{t} = t_x \underline{i} + t_y \underline{j} + t_z \underline{k}$$

(P.12)

$$N = \underline{t} \cdot \underline{n}$$

$$= \sigma_{xx} n_x^2 + \sigma_{yy} n_y^2 + \sigma_{zz} n_z^2$$

$$+ 2\sigma_{xy} n_x n_y + 2\sigma_{xz} n_x n_z + 2\sigma_{yz} n_y n_z$$

2a



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In index (ie $x \rightarrow 1, y \rightarrow 2, z \rightarrow 3$) notation

$$N = \underbrace{\left(\sum_{j,i=1}^3 \sigma_{ij} n_i e_j \right)}_{\underline{t}} \cdot \underbrace{\left(\sum_{k=1}^3 n_k e_k \right)}_{\underline{n}}$$

$$\sum_{i,j=1}^3 \sigma_{ij} n_i n_j$$

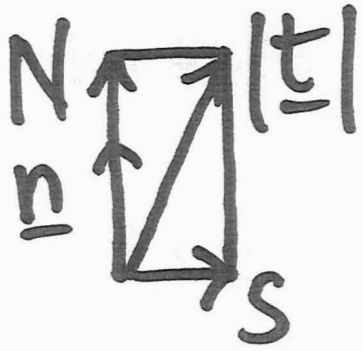
$[\sigma]$ $\{n\}$ \rightarrow column vector

(2L)

$$\Rightarrow N = \{n\}^T [\sigma] \{n\}$$

\rightarrow Matrix-vector notation.

\rightarrow (2C)



$$S^2 = |t_{(n)}|^2 - N^2$$

→ (n shown for clarity, can omit.)



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Ex 1 At $P(1, 3, 2)$

$$\underline{\underline{\sigma}} = \begin{bmatrix} x^2 & z & x \\ & xy & y \\ \text{symm} & & x^2 z \end{bmatrix}$$

On plane $\underline{n} = \frac{1}{\sqrt{3}} \underline{i} - \frac{1}{\sqrt{6}} \underline{j} + \frac{1}{\sqrt{2}} \underline{k}$

find \underline{t} , \underline{N} , \underline{S} . \rightarrow use ①, ②, ③ (p13, 17-19)

$$\underline{t} = \{t\} = [\underline{\sigma}] \{n\} = \begin{bmatrix} 1 & 2 & 1 \\ 2 & 3 & 3 \\ 1 & 3 & 2 \end{bmatrix} \begin{Bmatrix} 1/\sqrt{3} \\ -1/\sqrt{6} \\ 1/\sqrt{2} \end{Bmatrix} = \begin{Bmatrix} 0.47 \\ 2.05 \\ 0.77 \end{Bmatrix}$$

$$\underline{t} = 0.47 \underline{i} + 2.05 \underline{j} + 0.77 \underline{k}$$



For N , can use $2a$ or $2b$ or $2c$
(P17, 18)

Using $2b$,

$$N = 1 \cdot \frac{1}{3} + 2 \cdot 2 \cdot \frac{1}{\sqrt{3}} \left(-\frac{1}{\sqrt{6}} \right)$$

$$+ 2 \cdot 1 \cdot \frac{1}{\sqrt{3}} \cdot \frac{1}{\sqrt{2}} + 3 \cdot \frac{1}{6} + 2 \cdot 3 \left(-\frac{1}{\sqrt{6}} \right) \left(\frac{1}{\sqrt{2}} \right) + \frac{2}{2}$$

$$= -0.025 \quad \blacktriangledown \quad \text{(ie. acts along inward normal to plane)}$$

or simply do $\underline{t} \cdot \underline{n} = \{t\}^T \{n\}$

$$S = \pm \sqrt{|\underline{t}|^2 - N^2} = 2.2396 \text{ directed along } (\underline{n} \times \underline{e}(\underline{t})) \times \underline{n}$$

Note $|\underline{t}| \approx S \quad \because \quad N \approx 0$
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Lecture 3

STRESS ANALYSIS

- Transformation of $\underline{\underline{\sigma}}$
- Transf. of vector
- Introduction to 3D Cartesian Tensors.



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Transformation of Stress

Tensor $\underline{\underline{\sigma}}$



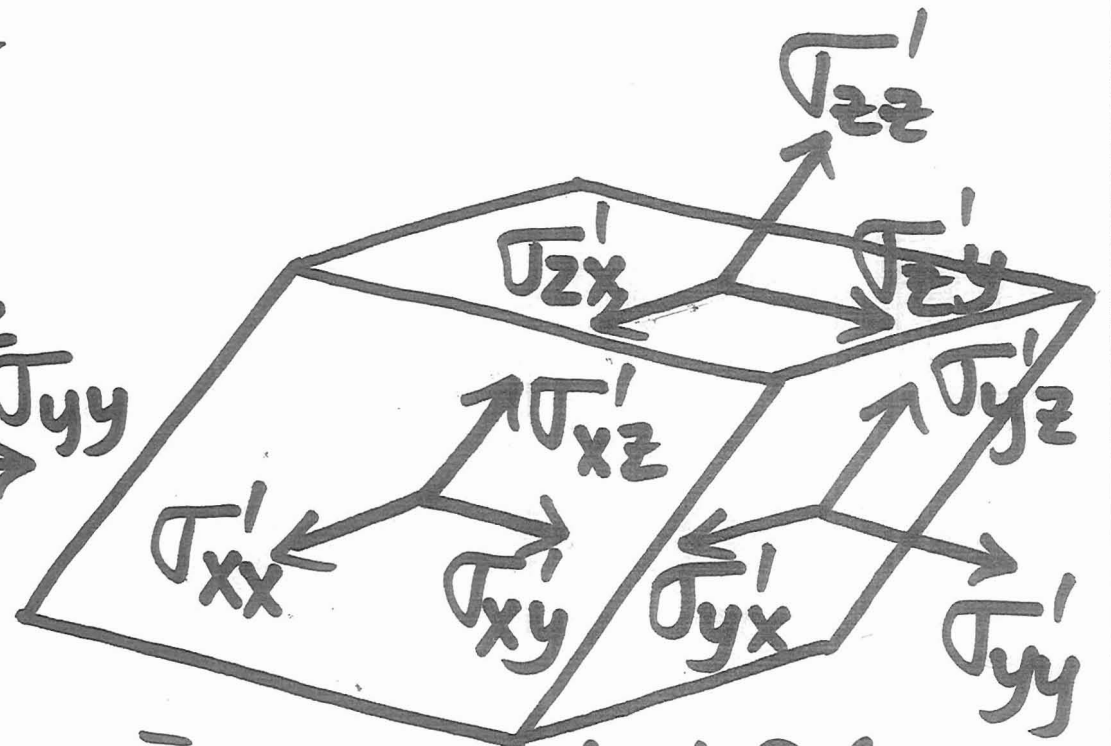
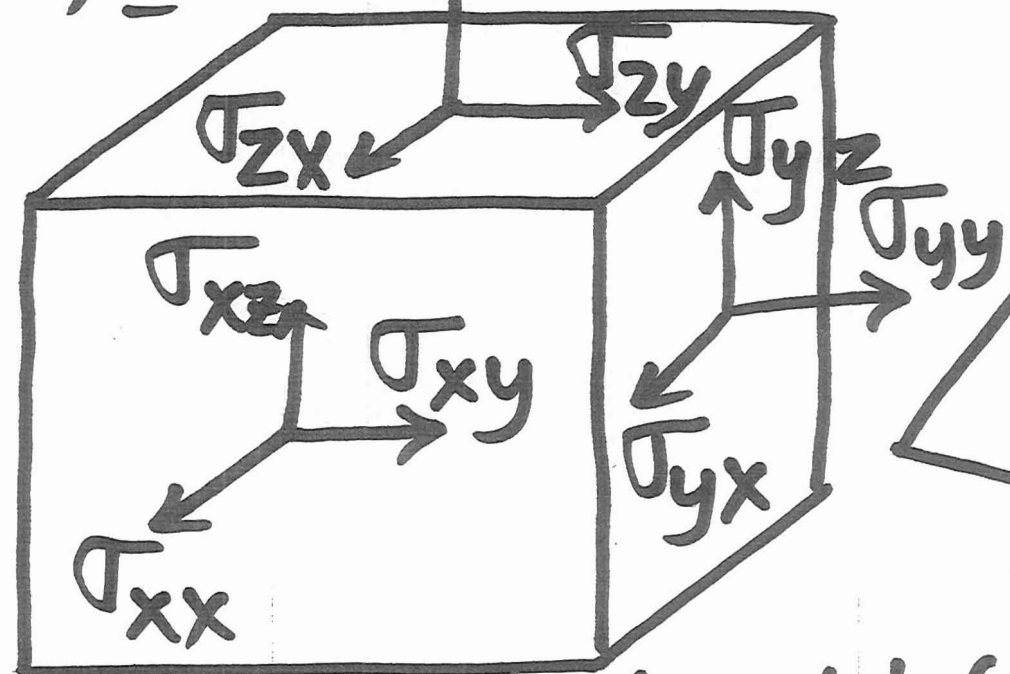
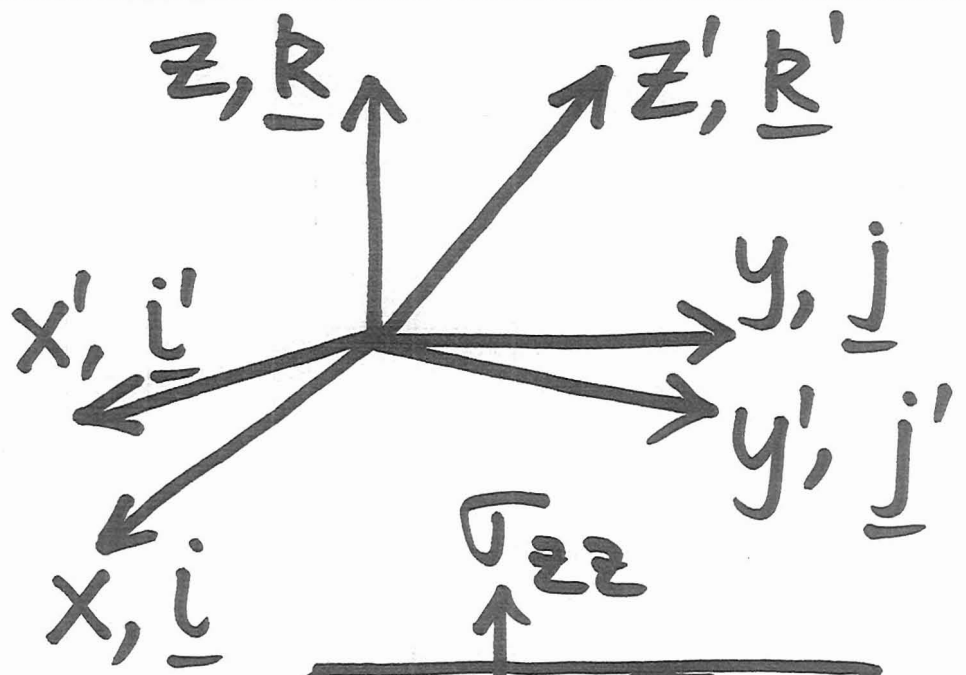
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Given $\underline{\underline{\sigma}}$ at P in (x, y, z) system

Find $\underline{\underline{\sigma'}}$ at P, ie stress tensor in (x', y', z') system.



L3



$\underline{\underline{\sigma}}$ at P (referred to (x, y, z)) ; $\underline{\underline{\sigma'}}$ at P (ref to (x', y', z'))

Transformation matrix (a)
 between (x, y, z) & (x', y', z')
 systems — matrix of direction
 cosines:



L3

	x, \underline{i}	y, \underline{j}	z, \underline{k}
x', \underline{i}'	$l_1 \equiv a_{11}$	$m_1 \equiv a_{12}$	$n_1 \equiv a_{13}$
y', \underline{j}'	$l_2 \equiv a_{21}$	$m_2 \equiv a_{22}$	$n_2 \equiv a_{23}$
z', \underline{k}'	$l_3 \equiv a_{31}$	$m_3 \equiv a_{32}$	$n_3 \equiv a_{33}$

a_{ij} is cosine of angle between x'_i / \underline{e}'_i & x_j / \underline{e}_j .

$x' \equiv x'_1, y' \equiv x'_2, z' \equiv x'_3, \underline{i}' \equiv \underline{e}'_1, \underline{j}' \equiv \underline{e}'_2, \underline{k}' \equiv \underline{e}'_3$

Need $\underline{\underline{\sigma}}'$ in terms of $\underline{\underline{\sigma}}$ & transformation matrix.



Diagonal (Normal) components.

σ_{xx}' = Normal stress on plane with \underline{i}' as normal.

$$\underline{i}' = l_1 \underline{i} + m_1 \underline{j} + n_1 \underline{k} = a_{11} \underline{i} + a_{12} \underline{j} + a_{13} \underline{k}$$

$$\Rightarrow \sigma_{xx}' = \sigma_{xx} l_1^2 + \sigma_{yy} m_1^2 + \sigma_{zz} n_1^2$$

$$+ 2\sigma_{xy} l_1 m_1 + 2\sigma_{yz} m_1 n_1 + 2\sigma_{xz} l_1 n_1$$

(ref (2a) (2b))
(L2, P.17, 18)

In index notation,

$$\sigma_{11}' = \sigma_{11} a_{11}^2 + \sigma_{22} a_{12}^2 + \sigma_{33} a_{13}^2 + 2\sigma_{12} a_{11} a_{12} + 2\sigma_{23} a_{12} a_{13} + 2\sigma_{13} a_{11} a_{13}$$



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Similarly $\sigma_{yy}' =$ Normal stress on j' plane
 $\sigma_{zz}' =$ " " " k' " "

$\sigma_{yy}' =$ (1a) with $l_1 \rightarrow l_2, m_1 \rightarrow m_2, n_1 \rightarrow n_2$
 $\sigma_{22}' =$ (1b) with 1 \rightarrow 2 in first subscript of a_{ij}
 $\sigma_{zz}' =$ (1a), $l_1 \rightarrow l_3, m_1 \rightarrow m_3, n_1 \rightarrow n_3$; $\sigma_{33}' =$ (1b), 1st subscr of $a_{ij}, 1 \rightarrow 3$.

Off-diagonal (Shear) Components

Ref. Fig P.2.

$\underline{t}_{(j')}$ = stress vector on \underline{j}' plane.

$$\underline{\tau}_{yz} = \underline{t}_{(j')} \cdot \underline{k}' = \underline{t}_{(k')} \cdot \underline{j}'$$

From transf. matrix

$$\underline{j}' = l_2 \underline{i} + m_2 \underline{j} + n_2 \underline{k}; \quad \underline{k}' = l_3 \underline{i} + m_3 \underline{j} + n_3 \underline{k}$$

$$\underline{t}_{(j')} = (\sigma_{xx} l_2 + \sigma_{xy} m_2 + \sigma_{xz} n_2) \underline{i} + (\sigma_{xy} l_2 + \sigma_{yy} m_2 + \sigma_{yz} n_2) \underline{j} + (\sigma_{xz} l_2 + \sigma_{yz} m_2 + \sigma_{zz} n_2) \underline{k}$$



$$\Rightarrow \sigma'_{yz} = \sigma_{xx} l_2 l_3 + \sigma_{yy} m_2 m_3 + \sigma_{zz} n_2 n_3 + \sigma_{xy} (m_2 l_3 + m_3 l_2) + \sigma_{xz} (n_2 l_3 + n_3 l_2) + \sigma_{yz} (n_2 m_3 + n_3 m_2)$$



index not...

↳ (2a)

$$\sigma'_{23} = \sigma_{11} a_{21} a_{31} + \sigma_{22} a_{22} a_{32} + \sigma_{33} a_{23} a_{33} + \sigma_{13} (a_{23} a_{31} + a_{33} a_{21}) + \sigma_{23} (a_{23} a_{32} + a_{33} a_{22}) + \sigma_{12} (a_{22} a_{31} + a_{32} a_{21})$$

↳ (2b)

Similarly, $\sigma_{xz}' = t_{(i')} \cdot \underline{k}' = t_{(k')} \cdot \underline{l}'$

$$\sigma_{xy}' = t_{(i')} \cdot \underline{j}' = t_{(j')} \cdot \underline{l}'$$



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$\sigma_{xz}' = \textcircled{2a}$ with $l_2 \rightarrow l_1; m_2 \rightarrow m_1; n_2 \rightarrow n_1$

$\sigma_{13}' = \textcircled{2b}$ with $a_{21} \rightarrow a_{11}; a_{22} \rightarrow a_{12}; a_{23} \rightarrow a_{13}$

$\sigma_{xy}' = \textcircled{2a}$ with $l_3 \rightarrow l_1; m_3 \rightarrow m_1; n_3 \rightarrow n_1$

$\sigma_{12}' = \textcircled{2b}$ with $a_{31} \rightarrow a_{11}; a_{32} \rightarrow a_{12}; a_{33} \rightarrow a_{13}$



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$\underline{\underline{\sigma}}$ ' in index notation (1b), (2b) etc) in compact form:

$$\sigma_{ij}' = \sum_{r=1}^3 \sum_{s=1}^3 a_{ir} a_{js} \sigma_{rs} \rightarrow (3a)$$

↳ Transf Law to get $\underline{\underline{\sigma}}$ ' from $\underline{\underline{\sigma}}$, given

$$= \sum_{r,s=1}^3 (a_{ir} \sigma_{rs}) (a_{sj})^T = (\underline{\underline{a}} \underline{\underline{\sigma}}) (\underline{\underline{a}}^T)_{i \quad j}$$

$$\underline{\underline{\sigma}}' = \underline{\underline{a}} \underline{\underline{\sigma}} \underline{\underline{a}}^T \rightarrow (3b)$$

$\underline{\underline{a}}^T$
 $\underline{\underline{P}} = P_{is} b_{sj}$

Note: $\underline{\underline{a^{-1}}} = \underline{\underline{a^T}}$, ie $\underline{\underline{aa^T}} = \underline{\underline{I}}$



Proof: $\underline{\underline{i'}} = a_{11}\underline{\underline{i}} + a_{12}\underline{\underline{j}} + a_{13}\underline{\underline{k}}$

ie 1st row of $\underline{\underline{a}}$ is $\underline{\underline{i'}}$, 2nd is $\underline{\underline{j'}}$, 3rd is $\underline{\underline{k'}}$

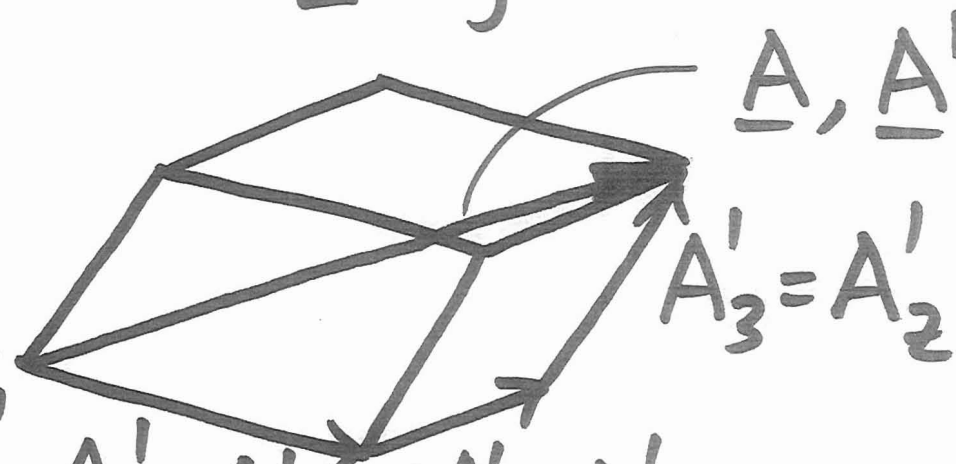
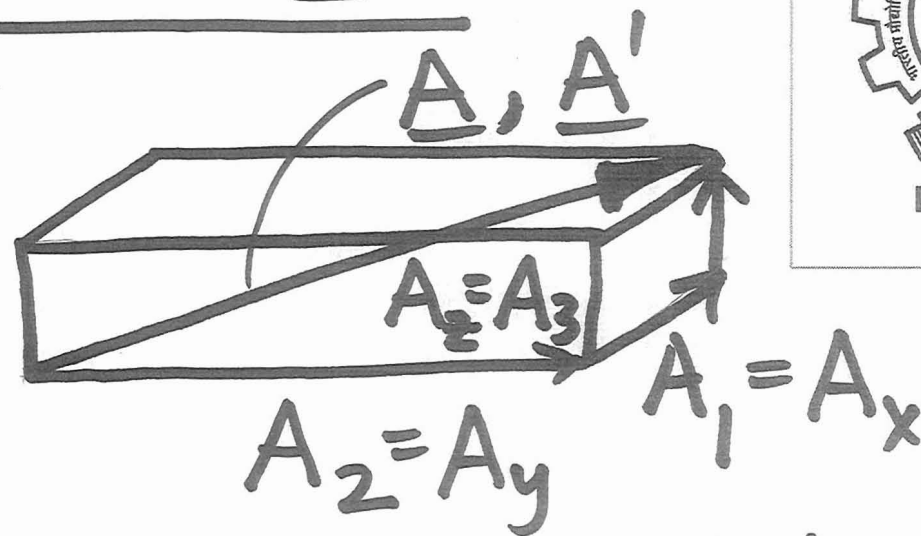
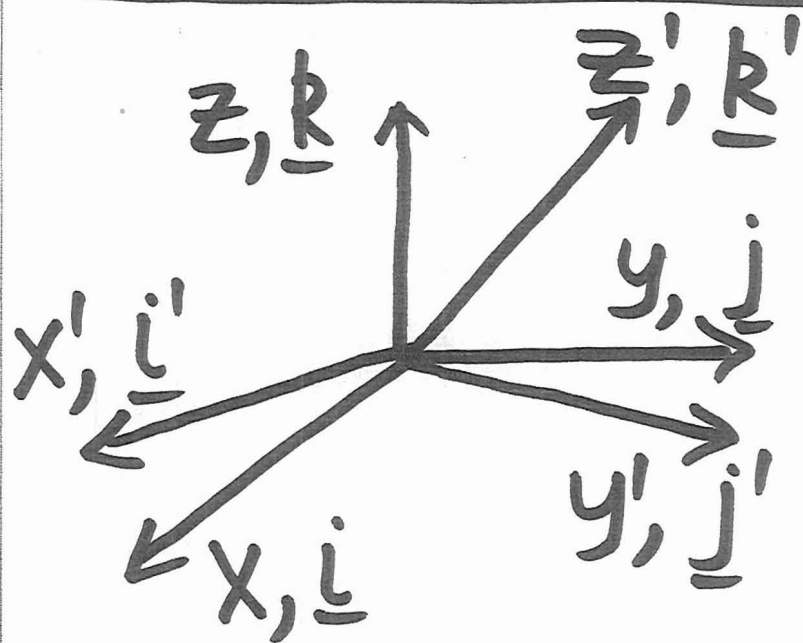
$$\Rightarrow \underline{\underline{aa^T}} = \begin{bmatrix} \underline{\underline{i'}} \rightarrow \\ \underline{\underline{j'}} \rightarrow \\ \underline{\underline{k'}} \rightarrow \end{bmatrix} \begin{bmatrix} \underline{\underline{i'}} \\ \underline{\underline{j'}} \\ \underline{\underline{k'}} \end{bmatrix} = \underline{\underline{I}}$$

$$\therefore \underline{\underline{i' \cdot i'}} = 1, \underline{\underline{i' \cdot j'}} = 0, \text{ etc}$$

$$\Rightarrow \underline{\underline{a^T}} \textcircled{3b} \underline{\underline{a}} \Rightarrow \underline{\underline{\sigma}} = \underline{\underline{a^T}} \underline{\underline{\sigma'}} \underline{\underline{a}} \rightarrow \textcircled{3c}$$

↳ Inverse transf.

Transf. of vector \underline{A}



$$\underline{A} = \underline{A}'$$

Given: $\underline{A} = A_1 \underline{e}_1 + A_2 \underline{e}_2 + A_3 \underline{e}_3$

Find: $\underline{A}' = A'_1 \underline{e}'_1 + A'_2 \underline{e}'_2 + A'_3 \underline{e}'_3$

Note $\underline{e}_1 \equiv \underline{i}$ $\underline{e}_2 \equiv \underline{j}$ $\underline{e}_3 \equiv \underline{k}$

$\underline{e}'_1 = \underline{i}$ $\underline{e}'_2 = \underline{j}'$ $\underline{e}'_3 = \underline{k}'$

$$\underline{A}' = \underline{A} = A_1 \underline{i} + A_2 \underline{j} + A_3 \underline{k}$$

Subst $\underline{i}, \underline{j}, \underline{k}$ in terms of $\underline{i}', \underline{j}', \underline{k}'$ from \underline{a} , ie $\underline{i} = a_{11} \underline{i}' + a_{21} \underline{j}' + a_{31} \underline{k}'$, etc

$$\begin{aligned} \Rightarrow \underline{A}' &= (a_{11} A_1 + a_{12} A_2 + a_{13} A_3) \underline{i}' \\ &+ (a_{21} A_1 + a_{22} A_2 + a_{23} A_3) \underline{j}' \\ &+ (a_{31} A_1 + a_{32} A_2 + a_{33} A_3) \underline{k}' \\ &= A'_1 \underline{i}' + A'_2 \underline{j}' + A'_3 \underline{k}' \end{aligned}$$



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L3

$$\Rightarrow A'_i = \sum_{r=1}^3 a_{ir} A_r \rightarrow (4a)$$

$$\begin{Bmatrix} A'_1 \\ A'_2 \\ A'_3 \end{Bmatrix} = \underline{A'} = \underline{a} \underline{A} = \underline{a} \begin{Bmatrix} A_1 \\ A_2 \\ A_3 \end{Bmatrix} \rightarrow (4b)$$



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Introduction to 3D Cartesian Tensors



1) Range convention: subscript unrepeated index, in a term, can take values 1, 2, or 3. It's called range/free index.

Summation convention: (due to Einstein) Index/subscript appearing twice (and only twice), in a term, implies summation over that index for index ranging from 1 to 3.

(eg) $\sigma_{ij} \sigma_{rj}$ is a term in an Eqn

i, r , range indices, can take values 1, 2, or 3. So above

represents $3 \times 3 = 9$ terms in the 9 respective Eqns (ie the i, r^{th} eqn), ie

$\sigma_{1j} \sigma_{1j}, \sigma_{1j} \sigma_{2j}, \sigma_{1j} \sigma_{3j}, \sigma_{2j} \sigma_{1j}, \sigma_{2j} \sigma_{2j},$

$\sigma_{2j} \sigma_{3j}, \sigma_{3j} \sigma_{1j}, \sigma_{3j} \sigma_{2j}, \sigma_{3j} \sigma_{3j}$, appearing

in the $i=1, r=1$ eqn, $i=1, r=2$ eqn, 13, 21, 22, 23, 31, 32, 33, eqn, respectively.



Also, from \sum convention,

$$\sigma_{ij} \tau_{rj} = \sum_{j=1}^3 \sigma_{ij} \tau_{rj}$$

So each of the 9 terms represent (contain)
a sum of 3 terms, i.e., for example,

$\sigma_{2j} \tau_{3j}$ appearing in 2-3 eqn is

$$\rightarrow = \sigma_{21} \tau_{31} + \sigma_{22} \tau_{32} + \sigma_{23} \tau_{33}$$

$$\text{or } \sigma_{ij} \tau_{rj} = \sigma_{i1} \tau_{r1} + \sigma_{i2} \tau_{r2} + \sigma_{i3} \tau_{r3}$$

$$\# \sigma_{ij} \tau_{rj} = \sigma_{iR} \tau_{rR} = \sigma_{iS} \tau_{rS}, \text{ i.e.,}$$



Σ indices are dummy indices.

However, range indices must be same in each term of an equation, i.e.,

$$\sigma_{ijtt} + p_{ijn} = q_{ijm} + b_{ijss}, \quad \checkmark$$

$\sigma_{ijtt} + p_{rqtt} = q_{lmte} + b_{rstt}$ X
eqn not homogenous in range indices, but it should be so.



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L3

Thus,

$$A'_i = a_{ir} A_r \rightarrow \textcircled{4a}, p13, \Sigma \text{ dropped}$$

\rightarrow Transf of Vector.



$$\sigma'_{ij} = a_{ir} a_{js} \sigma_{rs} \rightarrow \textcircled{3a}, p. 9, \Sigma \text{ dropped}$$

\rightarrow Transf of σ

$$A'_{ijpq \dots} = a_{ir} a_{jc} a_{pd} a_{qe} \dots A_{bcde \dots}$$

'n' range indices

\rightarrow $\textcircled{5}$

\rightarrow Transf Law for n^{th} order tensor.

'n' range indices.

Thus: vector is 1st order tensor
 $\underline{\underline{\sigma}}$ is 2nd order tensor
 scalar is 0th order tensor



Also, $t_j = \sigma_{ij} n_i \rightarrow \textcircled{1}, \text{p.13, L2, } \Sigma \text{ omit}$
 $\rightarrow \text{Cauchy relation.}$

eg $j=2, t_2 = t_y = \sigma_{12} n_1 + \sigma_{22} n_2 + \sigma_{32} n_3$

$N = \sigma_{ij} n_i n_j \rightarrow \textcircled{2b}, \text{p18, L2, } \Sigma \text{ omit}$
 $\rightarrow \text{Normal stress.}$

$$S^2 = \underline{t} \cdot \underline{t} - N^2 = t_i t_i - N^2 = \sigma_{ji} n_j \sigma_{ki} n_k - (\sigma_{ij} n_i n_j)^2$$

$\underbrace{\begin{matrix} \text{Ti} & \text{N} & \text{Ti} \\ \text{Ti} & \text{N} & \text{Ti} \end{matrix}}_{\text{Ti}}$ $\underbrace{\begin{matrix} \text{Ti} & \text{N} & \text{Ti} \\ \text{Ti} & \text{N} & \text{Ti} \end{matrix}}_{\text{Ti}}$

$\underbrace{\begin{matrix} \text{Ti} & \text{N} & \text{Ti} \\ \text{Ti} & \text{N} & \text{Ti} \end{matrix}}_{\text{Ti}}$ $\underbrace{\begin{matrix} \text{Ti} & \text{N} & \text{Ti} \\ \text{Ti} & \text{N} & \text{Ti} \end{matrix}}_{\text{Ti}}$

19a

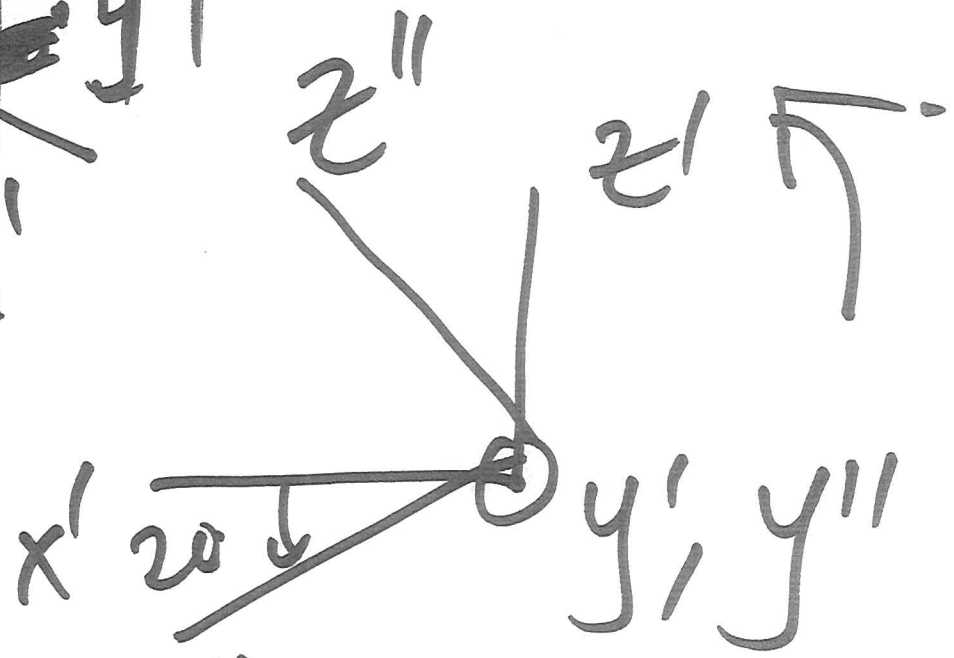
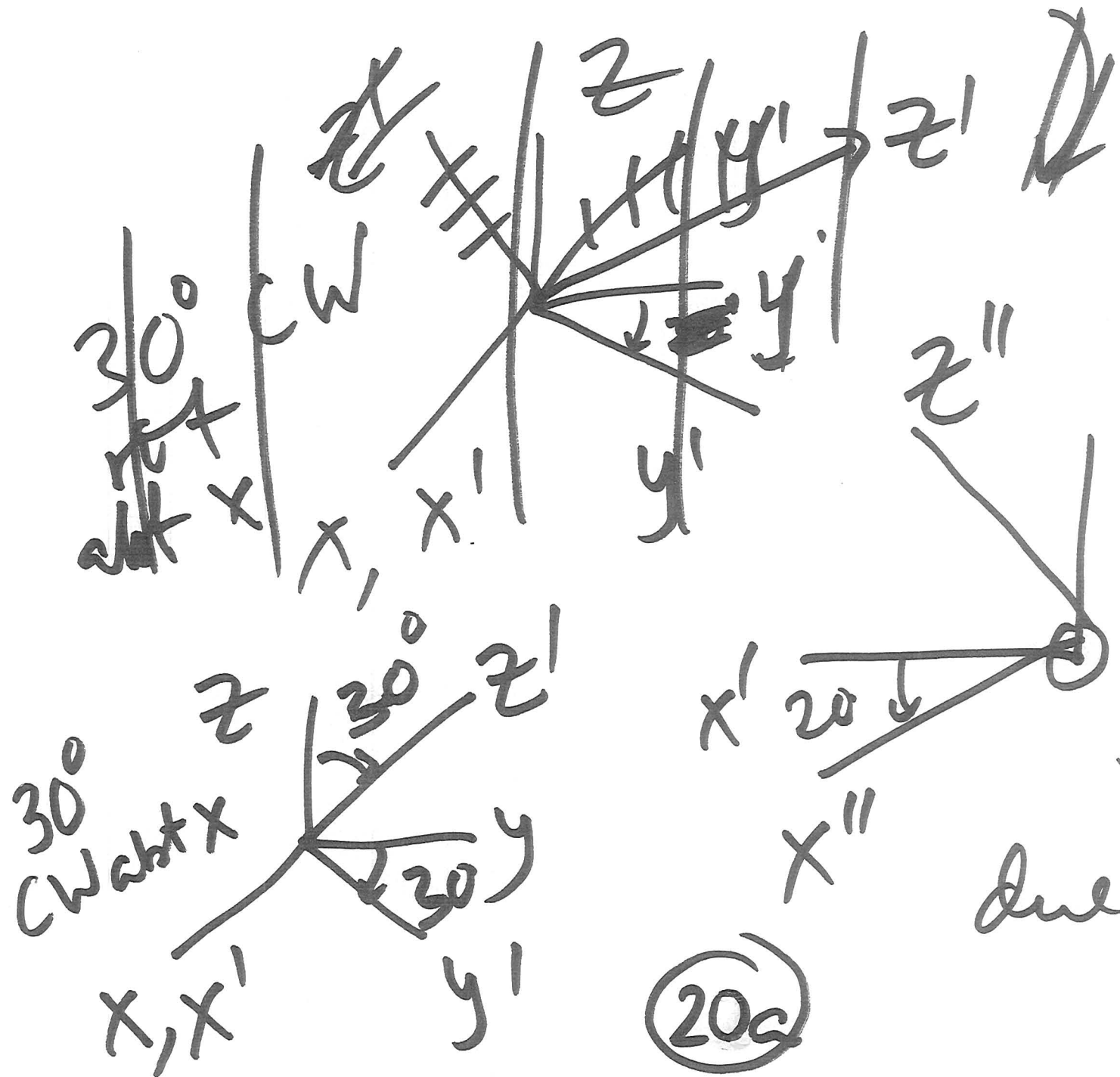
Ex 2 Given $\underline{\underline{\sigma}} = \begin{bmatrix} 10 & 20 & 30 \\ 20 & 5 & 25 \\ 30 & 25 & 15 \end{bmatrix}$ MPa at P



(x, y, z) rotated 30° CW about x (ie, -i rot)
 Then (x', y', z') rotated 20° CCW abt y'.
 (ie, +j' rot).

Find $\underline{\underline{\sigma}}''$.

$$\begin{Bmatrix} \underline{i}' \\ \underline{j}' \\ \underline{k}' \end{Bmatrix} = \begin{bmatrix} 1 & 0 & 0 \\ 0 & \cos 30 & -\sin 30 \\ 0 & \sin 30 & \cos 30 \end{bmatrix} \begin{Bmatrix} \underline{i} \\ \underline{j} \\ \underline{k} \end{Bmatrix} = \underline{\underline{a}}_I \begin{Bmatrix} \underline{i} \\ \underline{j} \\ \underline{k} \end{Bmatrix}$$



due to 20° CW rot abt y'

(20a)

$$\begin{Bmatrix} \underline{l}'' \\ \underline{j}'' \\ \underline{k}'' \end{Bmatrix} = \begin{bmatrix} \cos 20 & 0 & -\sin 20 \\ 0 & 1 & 0 \\ \sin 20 & 0 & \cos 20 \end{bmatrix} \begin{Bmatrix} \underline{l}' \\ \underline{j}' \\ \underline{k}' \end{Bmatrix} = \underline{a} \begin{Bmatrix} \underline{l}' \\ \underline{j}' \\ \underline{k}' \end{Bmatrix}$$



$$\Rightarrow \begin{Bmatrix} \underline{l}'' \\ \underline{j}'' \\ \underline{k}'' \end{Bmatrix} = \underbrace{\underline{a} \underline{a}^T}_{\underline{a}} \begin{Bmatrix} \underline{l}' \\ \underline{j}' \\ \underline{k}' \end{Bmatrix} = \begin{bmatrix} c20 & -s20s30 & -s20c30 \\ 0 & c30 & -s30 \\ s20 & c20s30 & c20c30 \end{bmatrix}$$

$$\underline{a} \underline{a}^T = \underline{a} \underline{\sigma} \underline{a}^T = \begin{bmatrix} 10.96 & 33.70 & 14.71 \\ 33.70 & 22.38 & 20.78 \\ 14.71 & 20.78 & -3.34 \end{bmatrix}$$

LECTURE 4

STRESS ANALYSIS



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- Intro to 3D Tensors (contd)
- Principal Stresses & Axes.

2) Kronecker delta

$$\delta_{ij} = 1, \text{ if } i=j$$
$$= 0, \text{ if } i \neq j$$

$\Rightarrow \delta_{ij} \equiv \underline{\underline{I}}$ (identity matrix).

Recall $\underline{\underline{a}} \underline{\underline{a}}^T = \underline{\underline{I}} \Rightarrow a_{ik} \cancel{b_{kj}} = \delta_{ij}$

$\Rightarrow \boxed{a_{ik} a_{jk} = \delta_{ij}}$ \rightarrow index notation form of $\underline{\underline{a}} \underline{\underline{a}}^T = \underline{\underline{I}}$

(eg) $A_{\nu} \delta_{uv} \overset{\text{for}}{=} A_1 \delta_{11} + A_2 \delta_{\cancel{1}2} + A_3 \delta_{\cancel{1}3} = A_1$





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$$\begin{aligned} \text{for } u=2, & A_v \delta_{uv} = A_2 \\ & u=3, & & = A_3 \end{aligned}$$

$$\Rightarrow A_v \delta_{uv} = A_u$$

$$\text{So, } \sigma_{ij} \delta_{jk} = \sigma_{ik} \quad ; \quad \left. \begin{aligned} \sigma_{ij} \delta_{ij} &= \sigma_{jj} \\ &= \sigma_{11} + \sigma_{22} + \sigma_{33} \end{aligned} \right|$$

3) Addition of tensors.

$$\begin{aligned} \underbrace{A_{ijk} + B_{ijk}}_{C_{ijk}} &= a_{ir} a_{js} a_{kt} (A_{rst} + B_{rst}) \\ &= a_{ir} a_{js} a_{kt} C_{rst} \end{aligned}$$

So adding like tensors (ie with same range indices) is allowed & it yields a tensor.



L4
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4) Tensor Multiplication.

a) Inner (dot) product of vectors

$$\underline{A} \cdot \underline{B} = A_i B_i$$

$$\left(\text{or } A_i \underline{e}_i \cdot B_j \underline{e}_j, \underline{e}_i \cdot \underline{e}_j \right) \Rightarrow A_i B_j \delta_{ij} = A_i B_i \quad \delta_{ij}$$

b) Cross product of vectors

$$\underline{A} \times \underline{B} = \underline{C} \rightarrow C_i = \epsilon_{ijk} A_j B_k$$

or

$$\underline{A} \times \underline{B} = A_j \underline{e}_j \times B_k \underline{e}_k$$

$$\underline{e}_j \times \underline{e}_k = \epsilon_{ijk} \underline{e}_i$$

$$\underline{C} = \underline{A} \times \underline{B} \Rightarrow \underbrace{C_i}_{=} \underbrace{\epsilon_{ijk} A_j B_k}_{=} \underline{e}_i$$



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where permutation symbol

$$\epsilon_{ijk} = 0, \text{ repeated indices} \\ (i=j, i=k, j=k)$$

$$= 1, \text{ ijk form}$$



$$\text{eg } \epsilon_{123} = 1$$

$$\text{eg } \epsilon_{312} = 1$$

$$= -1, \text{ ijk form}$$

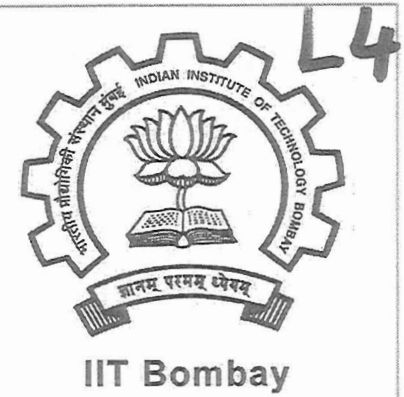


$$\text{eg } \epsilon_{132} = -1$$

$$\text{eg } C_2 = \epsilon_{213} A_1 B_3 + \epsilon_{231} A_3 B_1$$

$$= -A_1 B_3 + A_3 B_1 \text{ (as expected)}$$

$$\text{ie } C_y = -A_x B_z + A_z B_x$$



c) Outer product

eg $A_{ijk} B_{rs}$

d) Inner product

Outer product followed by contraction
(i.e., identifying two indices)

eg $A_{ijk} B_{rs} \xrightarrow{i=s} A_{ijk} B_{ri}$

outer product

Note: contraction lowers order of tensor by 2.



L4

$$A'_{ijk} B'_{rs} = a_{ib} a_{jc} a_{kd} a_{re} a_{sf} A_{bcdef}$$



$$= a_{jc} a_{kd} a_{re} A_{fcd} B_{ef} \quad \text{(QED)}$$

e) Differentiation of tensors:-

It increases order by 1.

$$\frac{\partial A'_{uv\dots}}{\partial x'_w} = \frac{\partial (a_{ui} a_{vj\dots} A_{ij\dots})}{\partial x_k} \frac{\partial x_k}{\partial x'_w}$$

Now, $x_k = a_{wk} x'_w$ (inv. transf of p.v.).

$$\Rightarrow \frac{\partial x_k}{\partial x'_w} = a_{wk},$$

$$\Rightarrow \frac{\partial A'_{uv\dots}}{\partial x'_w} = a_{ui} a_{vj} a_{wk} \dots \frac{\partial A_{ij\dots}}{\partial x_k} \quad (\text{QED})$$

f) Gradient of scalar $\phi(x_1, x_2, x_3)$

$$\underline{\nabla} \phi = \left(\frac{\partial (\)}{\partial x_i} \underline{e}_i \right) \phi = \frac{\partial \phi}{\partial x_i} \underline{e}_i = \phi_{,i} \underline{e}_i \equiv \phi_{,i}$$



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If $\phi(x_1, x_2, x_3) = \text{constant}$ defines surface S , and \underline{dx} is a differential tangent vector on S ,

$$d\phi = \nabla\phi \cdot \underline{dx} = 0 \quad (\text{on } S)$$

$\Rightarrow \nabla\phi$ is normal to S

$$= \phi_{,i} \rightarrow \phi_{,i} / |\phi_{,i}| = \text{unit normal}$$

9) Divergence of vector $\underline{A} = A_i \underline{e}_i$

$$\underline{\nabla} \cdot \underline{A} = \nabla_i A_i = \frac{\partial A_1}{\partial x_1} + \frac{\partial A_2}{\partial x_2} + \frac{\partial A_3}{\partial x_3}$$



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L4

or,

$$\underline{\nabla} \cdot \underline{A} = \nabla_i \underline{e}_i \cdot A_j \underline{e}_j = \nabla_i A_i$$

h) Curl of vector \underline{A}

$$\underline{C} = \underline{\nabla} \times \underline{A} = \epsilon_{ijk} \nabla_j A_k \underline{e}_i$$

$$= \underbrace{\epsilon_{ijk} A_{k,j}} \underline{e}_i = C_i \underline{e}_i$$

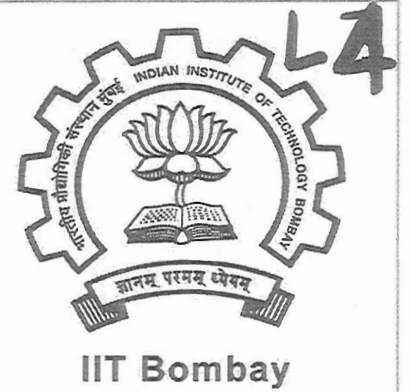
$$\equiv C_i$$

eg $i=2$, $C_2 = \epsilon_{231} A_{1,3} + \epsilon_{213} A_{3,1}$

$$= A_{1,3} - A_{3,1}$$



Principal Stresses, Principal Axes



Given : $\underline{\underline{\sigma}}$ at P in (x, y, z) system

Find : Planes \underline{n} on which \underline{t} is wholly normal, ie \underline{t} & \underline{n} have same direction.

Will be seen later that this is equivalent to finding planes \underline{n} on which N is an extremum (ref Shames, Engg Mech).

$$\underline{t} = \lambda \underline{n} \quad // \quad (\underline{t} \text{ 'wholly' along } \underline{n})$$

$$// \quad t_i = \lambda n_i = \lambda n_j \delta_{ij}$$



$$\underline{t} = \underline{\sigma} \underline{n} \quad // \quad t_i = \sigma_{ij} n_j \quad (\text{Cauchy rel})$$

$$\Rightarrow (\underline{\sigma} - \lambda \underline{I}) \underline{n} = \underline{0} \quad // \quad (\sigma_{ij} - \lambda \delta_{ij}) n_j = 0_i$$

①
}
→ Eigenvalue prob

3 Linear homogenous eqns

for 4 unknowns n_1, n_2, n_3, λ

Non-trivial soln, $\underline{n} \neq \underline{0}$ iff $\det | \underline{\sigma} - \lambda \underline{I} | = 0$

$$\det \underline{\underline{|\underline{\sigma} - \lambda \underline{\underline{I}}|}} = \lambda^3 - I_1 \lambda^2 + I_2 \lambda - I_3 = 0$$

where, $I_1 = \text{trace}(\underline{\underline{\sigma}}) = \sigma_{ii}$ ②

$$I_2 = \sigma_{11}\sigma_{22} + \sigma_{22}\sigma_{33} + \sigma_{33}\sigma_{11} \\ - \sigma_{12}^2 - \sigma_{23}^2 - \sigma_{13}^2$$

$$I_3 = \det \underline{\underline{|\underline{\sigma}|}}$$

Solve ② for $\lambda(1), \lambda(2), \lambda(3) \rightarrow$ evalues
 Subst evalue $\lambda(i)$ in ①, solve $\underline{n}(i) \rightarrow$ evector
 as follows:



$$(\sigma_{11} - \lambda(i))n_1 + \sigma_{12}n_2 + \sigma_{13}n_3 = 0$$

$$\sigma_{12}n_1 + (\sigma_{22} - \lambda(i))n_2 + \sigma_{23}n_3 = 0$$

$$\sigma_{13}n_1 + \sigma_{23}n_2 + (\sigma_{33} - \lambda(i))n_3 = 0$$



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3(a, b, c)

At most two of 3 (a, b, c) independent.

Use those two, &

$$n_1^2 + n_2^2 + n_3^2 = 1$$

→ 3d

ie, make n unit vector

& solve for $(n_1, n_2, n_3) \equiv \underline{n}(i) \rightarrow$ e vector corresponding to $\lambda(i)$

$\lambda(i) \rightarrow$ Principal Stresses (evalues)

$\underline{n}(i) \rightarrow$ Principal axes (e vectors)

ie, normal to planes
on which $\underline{t} = \lambda \underline{n}$



P-stresses are real:

Assume $\lambda = a + ib$, complex

$\therefore \underline{\sigma}$ real, polynomial has real coeffs.

$\Rightarrow \bar{\lambda}$ also a root (evalue).

$\Rightarrow (\lambda, \underline{n})$ & $(\bar{\lambda}, \bar{\underline{n}})$ are e-solutions
satisfying evp

$$\text{ie, } \underline{\sigma} \underline{n} = \lambda \underline{n} \rightarrow (a)$$

$$\underline{\sigma} \underline{\bar{n}} = \bar{\lambda} \underline{\bar{n}} \rightarrow (b)$$

$\underline{\bar{n}}^T * (a) - \underline{n}^T * (b)$ gives

$$\underbrace{\underline{\bar{n}}^T \underline{\sigma} \underline{n}} - \underbrace{\underline{n}^T \underline{\sigma} \underline{\bar{n}}} = \lambda \underbrace{\underline{\bar{n}}^T \underline{n}} - \bar{\lambda} \underbrace{\underline{n}^T \underline{\bar{n}}}$$

all scalars, so equal to their transpose.

$$\Rightarrow \underline{n}^T (\underline{\sigma}^T - \underline{\sigma}) \underline{\bar{n}} = (\lambda - \bar{\lambda}) \underline{n}^T \underline{\bar{n}}$$

$\therefore \underline{\sigma}$ symm $\Rightarrow \lambda = \bar{\lambda} = \text{real (QED)}$
for $\underline{n}, \underline{\bar{n}}, \neq \underline{0}$



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P-axes are orthogonal.

$$\underline{\underline{\sigma}} \underline{n}(i) = \lambda(i) \underline{n}(i) \rightarrow (c)$$

$$\underline{\underline{\sigma}} \underline{n}(j) = \lambda(j) \underline{n}(j) \rightarrow (d)$$

$\underline{n}^T(j) * (c) - \underline{n}^T(i) * (d)$ gives 'eigensolution.

(note all triple vector matrix products & vector [dot] products are scalars, and $\underline{\underline{\sigma}}$ symm, as before)

$$0 = (\lambda(i) - \lambda(j)) \underline{n}^T(i) \underline{n}(j)$$



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$\therefore \lambda(i) \neq \lambda(j)$, in general,

$$\underline{n}^T(i) \underline{n}(j) = 0 \quad (\text{Q.E.D.})$$



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For $\underline{\sigma}$ real, symm, even if $\lambda(i) = \lambda(j)$ (repeated roots) we can find e vectors $\underline{n}(i), \underline{n}(j)$ & orthogonalize them if reqd.

Thus if two p-stresses are same, then p-axis corresponding to third p-stress is uniquely determinable & other two p-axes are arbitrary

but orthogonal to 3rd p-axis,
& orthogonalizable wrt each
other. Thus for third [two
repeated] p-stresses two [one] of
3 (a, b, c) are independent.



If all p-stresses same, then any
three indep axes are p-axes.
This corresponds to case when only
one of 3 (a, b, c) are indep. However,
p-axes still orthogonalizable.

Ex1 Given

$$\underline{\underline{\sigma}} = \begin{bmatrix} 0 & 0 & -cx_2 \\ 0 & 0 & cx_1 \\ -cx_2 & cx_1 & 0 \end{bmatrix} \text{ at } P = (1, 2, 4)$$

Find p-stresses & axes.

$$I_1 = 0, I_2 = -c^2 \cdot 1^2 - c^2 \cdot 2^2 = -5c^2, I_3 = 0$$

$$\lambda^3 - 5c^2 \lambda = 0, \lambda = 0, \pm c\sqrt{5} \text{ (p-stresses)}$$

$$\underline{\underline{\lambda=0}}: \begin{cases} -2cn_3 = 0 \\ cn_3 = 0 \end{cases}$$

$$-2cn_1 + cn_2 = 0$$

after normalizing

$$\Rightarrow \underline{\underline{n(1)}} = \frac{+1}{\sqrt{5}} e_1 + \frac{+2}{\sqrt{5}} e_2$$

$$\lambda(1) = 0$$



$$\underline{\lambda = c\sqrt{5}}: -c\sqrt{5}n_1 - 2cn_3 = 0$$

$$(a) \leftarrow -c\sqrt{5}n_2 + cn_3 = 0$$

$$(c) \leftarrow -2cn_1 + cn_2 - c\sqrt{5}n_3 = 0$$

$$\underline{n(2)} = \pm \frac{(-2, 1, \sqrt{5})}{\sqrt{10}} = \pm \left(-\sqrt{\frac{2}{5}}\underline{e}_1 + \sqrt{\frac{1}{10}}\underline{e}_2 + \sqrt{\frac{1}{2}}\underline{e}_3 \right)$$

$$\underline{\lambda = -c\sqrt{5}}: \underline{n(3)} = \pm \left(\sqrt{\frac{2}{5}}\underline{e}_1 - \sqrt{\frac{1}{10}}\underline{e}_2 + \sqrt{\frac{1}{2}}\underline{e}_3 \right)$$

(by observation of above)

$$\text{See } \underline{n(1)} \cdot \underline{n(2)} = \underline{n(2)} \cdot \underline{n(3)} = \underline{n(3)} \cdot \underline{n(1)} = 0.$$



Ex2 Given,

$$\underline{\underline{D}} = \begin{bmatrix} 2 & 1 & -1 \\ 1 & 2 & -1 \\ -1 & -1 & 2 \end{bmatrix} \text{ at } P.$$

Find: p-solution.

$$\lambda^3 - 6\lambda^2 + 9\lambda - (2*3 - 1*1 - 1*1) = 0$$

→ 4.

$$\Rightarrow \lambda = 1, 1, 4$$

$\lambda = 4$:
$$\left. \begin{aligned} -2n_1 + n_2 - n_3 &= 0 \\ n_1 - 2n_2 - n_3 &= 0 \\ -n_1 - n_2 - 2n_3 &= 0 \end{aligned} \right\} \rightarrow \text{after normalizing,}$$

$$\underline{n(1)} = \frac{\pm (-1, -1, 1)}{\sqrt{3}}$$
$$= \pm (\underline{e}_1 + \underline{e}_2 - \underline{e}_3) / \sqrt{3}$$



$$\lambda=1: \begin{cases} n_1 + n_2 - n_3 = 0 \\ n_1 + n_2 - n_3 = 0 \\ -n_1 - n_2 + n_3 = 0 \end{cases} \left. \begin{array}{l} \text{all 3} \\ \text{eqns} \\ \text{same,} \\ \text{ie only} \end{array} \right\} \text{one indep.}$$



So we have freedom to choose 2 comp's of \underline{n} independently, ie to choose 2 indep e vectors or \underline{n} 's.

→ choose $n_2 = n_3 = 1$, ie $\underline{n}(2) = \frac{1}{\sqrt{2}}(\underline{e}_2 + \underline{e}_3)$

→ choose $n_1 = n_3 = 1 \Rightarrow \underline{n}(3) = \frac{1}{\sqrt{2}}(\underline{e}_1 + \underline{e}_3)$

$$\text{or, } \underline{n}(3) = \underline{n}(1) \times \underline{n}(2) \\ = \frac{1}{\sqrt{6}}(-2\underline{e}_1 + \underline{e}_2 - \underline{e}_3)$$



observe $\underline{n}(1) \cdot \underline{n}(2) = \underline{n}(1) \cdot \underline{n}(3) = 0$;

$\underline{n}(1), \underline{n}(2), \underline{n}(3)$, indep;

$$\underline{n}(2) \cdot \underline{n}(3) = 0$$

→ second version (this pg)

LECTURE 5.

STRESS ANALYSIS



L5

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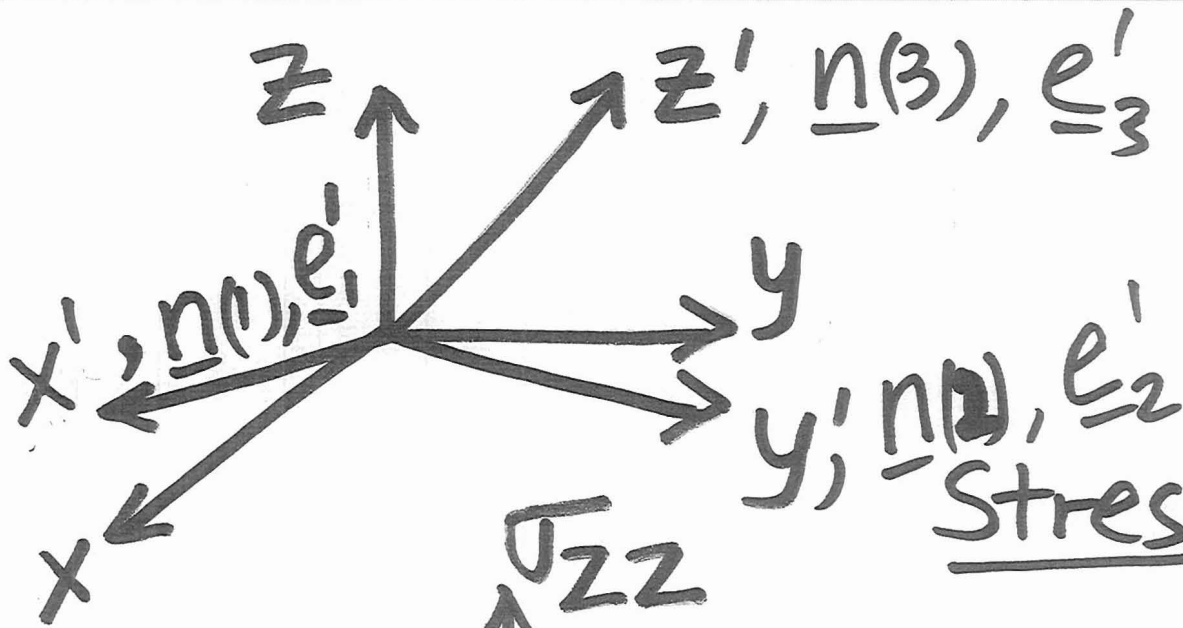
- $\underline{\underline{\sigma}}$, $\underline{\underline{t}}$, N , S referred to p-system.
- Invariants of Stress
- 2-D p-stress theory.
- Octahedral Stresses.

$\underline{\underline{\sigma}}, \underline{\underline{t}}, N, S,$ referred to
principal coordinate system

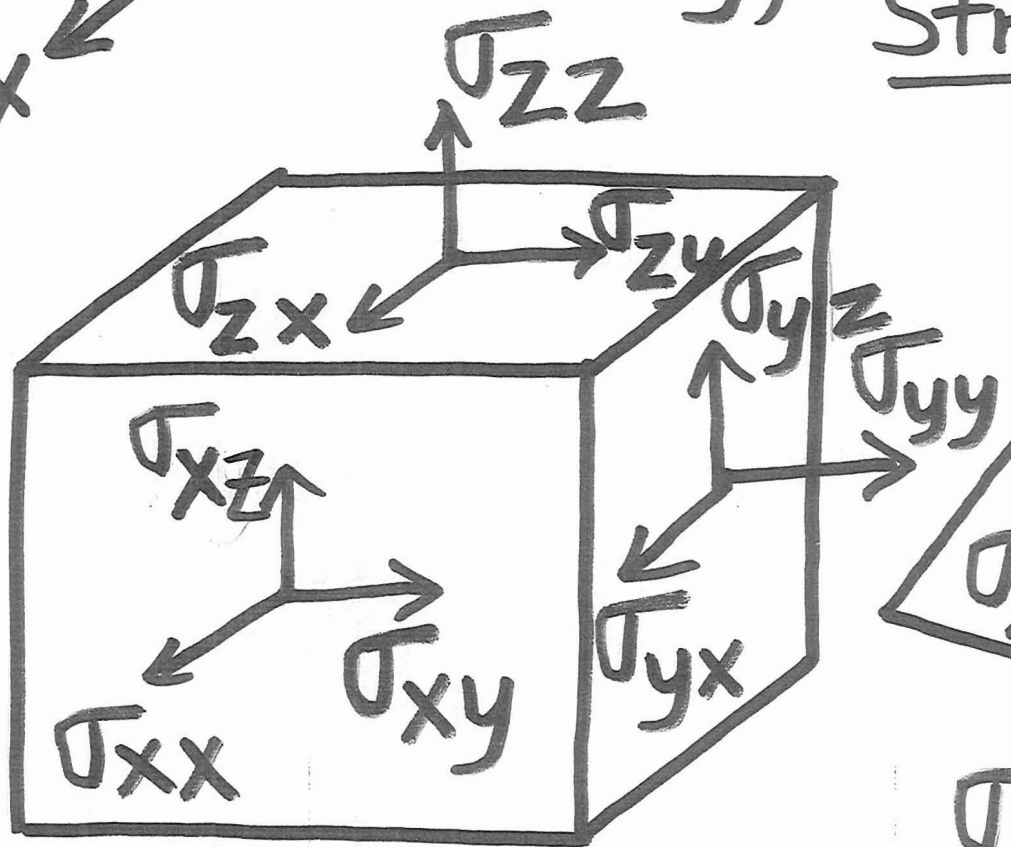


Consider x', y', z' aligned along p-axes
 $\underline{n(1)}, \underline{n(2)}, \underline{n(3)},$ respectively. Thus we
have a p-coord system. This
pertains to pt. P.

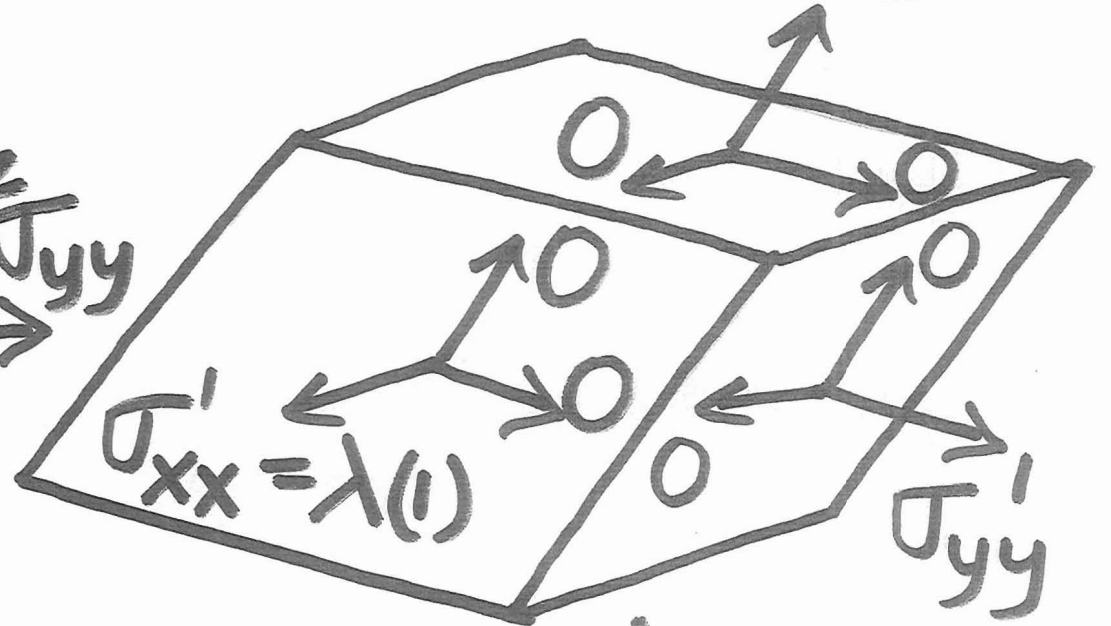
$$\Rightarrow \underline{\underline{\sigma}}' = \begin{bmatrix} \lambda(1) & 0 & 0 \\ 0 & \lambda(2) & 0 \\ 0 & 0 & \lambda(3) \end{bmatrix} = \begin{bmatrix} \sigma_{x'x'} & 0 & 0 \\ 0 & \sigma_{y'y'} & 0 \\ 0 & 0 & \sigma_{z'z'} \end{bmatrix}$$



Stresses at P.



$\underline{\underline{\sigma}}$ in x, y, z



$\underline{\underline{\sigma}}$ in P-system x', y', z'

Consider arbitrary plane defined by normal

$$\underline{n} = n_i \underline{e}_i = \underline{n}' = n'_i \underline{e}'_i$$

$$\Rightarrow N = \{\underline{n}'\}^T [\underline{\sigma}'] \{\underline{n}'\} = \sigma'_{ij} n'_i n'_j$$
$$= \underbrace{\sigma'_{11}}_{\lambda(1)} (n'_1)^2 + \underbrace{\sigma'_{22}}_{\lambda(2)} (n'_2)^2 + \underbrace{\sigma'_{33}}_{\lambda(3)} (n'_3)^2$$

$$N = \sum_{i=1}^3 \lambda(i) (n'_i)^2$$

(Σ convention suppressed).



Order, $\lambda(1) > \lambda(2) > \lambda(3)$
 (λ distinct)

$\Rightarrow N_{\max} = \lambda(1)$, for $n'_1=1, n'_2=n'_3=0$, ie $\underline{n}(1)$

$N_{\min} = \lambda(3)$, for $n'_3=1, n'_1=n'_2=0$, ie $\underline{n}(3)$

$\Rightarrow N$ extremum on p -planes.

$$\underline{t} = [\underline{\sigma}'] \{ \underline{n}' \} = \sigma'_{ij} n'_j \underline{e}_i$$

$$\underline{t} = \sum_{i=1}^3 \lambda(i) n'_i \underline{e}_i \quad (\Sigma \text{ conv suppressed}).$$



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$$S^2 = |\underline{t}|^2 - N^2$$

$$S = \left\{ (\lambda(1)n_1')^2 + (\lambda(2)n_2')^2 + (\lambda(3)n_3')^2 - (\lambda(1)(n_1')^2 + \lambda(2)(n_2')^2 + \lambda(3)(n_3')^2) \right\}^{27/2}$$



Invariants of Stress.

I_1, I_2, I_3 , are invariant with coord system. Hence p-stresses/axes invariant.

Proof:

$$I_1 = \sigma_{ii}$$

$$\sigma_{ij}^i = \underbrace{a_{ir} a_{js}^i}_{= \delta_{rs}} \sigma_{rs} = \sigma_{ss} \quad (\text{Q.E.D.}) \quad (\text{ie do contraction, use } \underline{\underline{a a^T = I}})$$





$$\underline{\underline{\sigma}}' = \underline{\underline{a}} \underline{\underline{\sigma}} \underline{\underline{a}}^T$$

$$I_3' = \det |\underline{\underline{\sigma}}'| = \det |\underline{\underline{a}}| \det |\underline{\underline{\sigma}}| \det |\underline{\underline{a}}^T|$$

$\nearrow I_3$

$\ast \det |\underline{\underline{a}}^T| \dots$

$$= \det |\underline{\underline{a}} \underline{\underline{a}}^T| I_3$$

$$= I_3 \quad (\underline{\underline{QED}})$$

$$I_2 = \frac{1}{2} \left(\underbrace{\sigma_{ii}}_{I_1} \underbrace{\sigma_{jj}}_{I_1} - \sigma_{ij} \sigma_{ij} \right) \quad \text{— expand to verify it is same as on p.12, L4}$$

$$I_2' = \frac{1}{2} (\sigma_{ii}' \sigma_{jj}' - \sigma_{ij}' \sigma_{ji}')$$

$$(\mathbf{I}_1')^2 = (\mathbf{I}_1)^2$$

$$\sigma_{ij}' \sigma_{kl}' = a_{ir} a_{js} a_{kt} a_{lu} \sigma_{rs} \sigma_{tu}$$

δ_{rt} δ_{su}

$$= \sigma_{rs} \sigma_{rs}$$

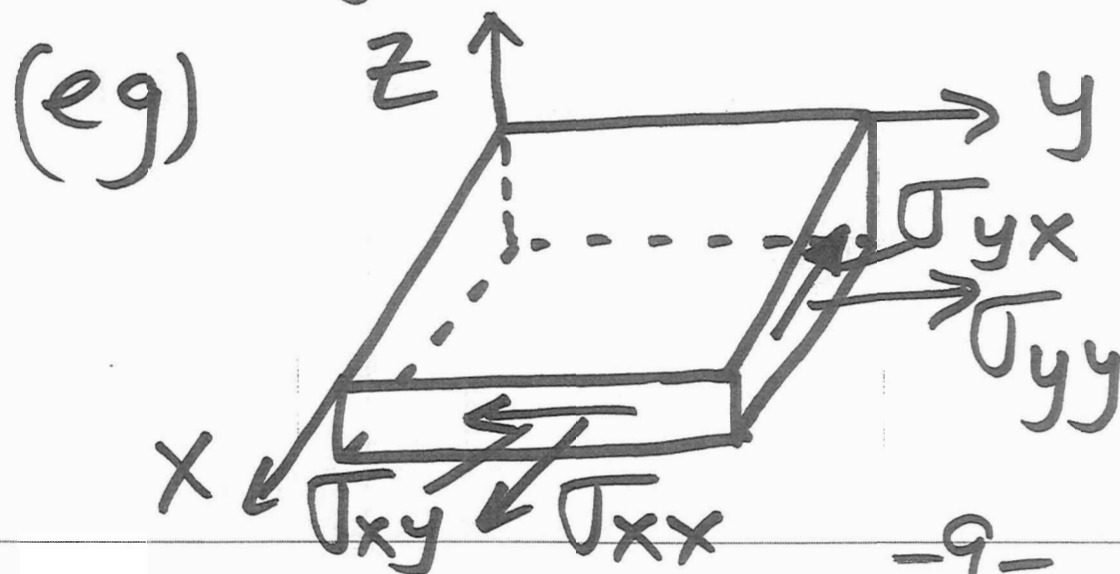
$$\Rightarrow I_2' = I_2 \quad (\text{Q.E.D.})$$



Principal Stresses for 2D State of Stress.



$$\sigma_{ij} = \begin{bmatrix} \sigma_{11} & \sigma_{12} & 0 \\ \sigma_{12} & \sigma_{22} & 0 \\ 0 & 0 & 0 \end{bmatrix}, \quad \sigma_{i3} = 0$$



Thin plate with inplane edge loads which are functions of (x, y) .

$$I_1 = \sigma_{11} + \sigma_{22}, I_2 = \sigma_{11}\sigma_{22} - \sigma_{12}^2$$

$$I_3 = 0$$

$$\lambda^3 - (\sigma_{11} + \sigma_{22})\lambda^2 + (\sigma_{11}\sigma_{22} - \sigma_{12}^2)\lambda = 0$$

$$\lambda = 0, \frac{\sigma_{11} + \sigma_{22}}{2} \pm \sqrt{\left(\frac{\sigma_{11} - \sigma_{22}}{2}\right)^2 + \sigma_{12}^2}$$

$\lambda(1) = 0$: $\sigma_{11}n_1 + \sigma_{12}n_2 = 0 \rightarrow$ (a) \leftarrow (-) for $\lambda(2)$
 $\sigma_{12}n_1 + \sigma_{22}n_2 = 0 \rightarrow$ (b) \leftarrow (+) for $\lambda(3)$
 $0 = 0 \rightarrow$ (c)

If $\begin{vmatrix} \sigma_{11} & \sigma_{12} \\ \sigma_{12} & \sigma_{22} \end{vmatrix} \neq 0$, $(n_1 = n_2 = 0, n_3 = 1)$



$$\text{If } \begin{vmatrix} \sigma_{11} & \sigma_{12} \\ \sigma_{12} & \sigma_{22} \end{vmatrix} = 0 = \sigma_{11}\sigma_{22} - \sigma_{12}^2$$

$$= I_2$$

$\Rightarrow \lambda = 0$ is double root.

ie (a), (b) dependent. $\lambda(1) = \lambda(2)$

(a) $\Rightarrow n_2 = -\frac{\sigma_{11}}{\sigma_{12}} n_1$ (eg) $(0, 0, 1)$

(c) $\Rightarrow n_3 = \text{arbitrary}$ & $(1, -\frac{\sigma_{11}}{\sigma_{12}}, 1)$

ie Choose arbitrary n_1, n_3 , ie 2 e vectors & then normalize.

$\lambda(2), \lambda(3)$: get e vectors in usual manner
 -11- ($I_2 \neq 0 \Rightarrow \lambda(2) \neq 0$).



$$(\sigma_{11} - \lambda(z))n_1(z) + \sigma_{12}n_2(z) = 0$$

$$\sigma_{12}n_1(z) + (\sigma_{22} - \lambda(z))n_2(z) = 0$$

$$-\lambda(z)n_3(z) = 0$$

$$\Rightarrow n_3(z) = 0$$

$$1 = (n_1(z))^2 + \left(\frac{\sigma_{11} - \lambda(z)}{\sigma_{12}}\right)^2 (n_1(z))^2 \quad \text{indep.}$$

$$n_1(z) = \pm \frac{\sigma_{12}}{[\sigma_{12}^2 + (\sigma_{11} - \lambda(z))^2]^{1/2}}; \quad n_2(z) = \mp \frac{(\sigma_{11} - \lambda(z))}{[\sigma_{12}^2 + (\sigma_{11} - \lambda(z))^2]^{1/2}}$$



use here → only one of these two indep. :-

Further on $\lambda(1) = \lambda(2) = 0$

For $\underline{n}_1, \underline{n}_3$ chosen as on p. 11,

$$\underline{n}(1) = (0, 0, 1)$$

$$\underline{n}(2) = \left(1, -\frac{\sigma_{11}}{\sigma_{12}}, 1 \right) / \sqrt{2 + \left(\frac{\sigma_{11}}{\sigma_{12}} \right)^2}$$

use $\lambda(3) = \sigma_{11} + \sigma_{22}$

$$\underline{n}(1) \cdot \underline{n}(3) = 0$$

$$\underline{n}(2) \cdot \underline{n}(3) = \left(\pm \sigma_{12} \mp \frac{\sigma_{11} \sigma_{22}}{\sigma_{12}} \right) / (\text{some denom})$$

So $\underline{n}(1), \underline{n}(2)$ always in plane \perp to \underline{n}_3
12(a)

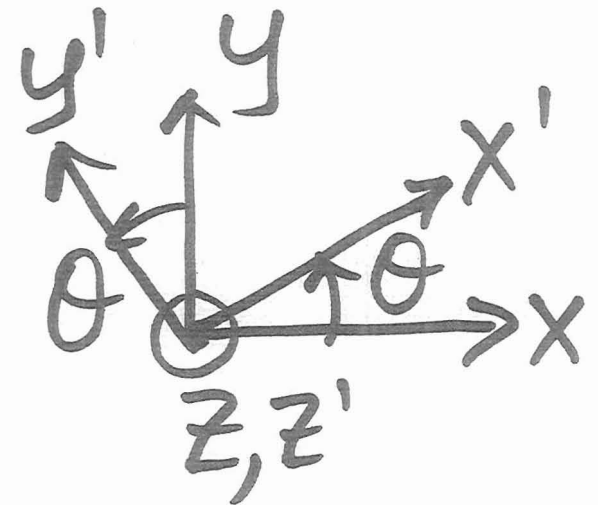


So whether $\lambda(z) \neq 0$ (unrepeated)
or $\lambda(z) = 0$ (repeated roots),
 $\underline{n}(z) = (0, 0, 1)$ is valid evector.



\Rightarrow Transf to p-system is rot. in
 x, x_2 plane, i.e.,

$$\underline{a} = \begin{bmatrix} \cos \theta & \sin \theta & 0 \\ -\sin \theta & \cos \theta & 0 \\ 0 & 0 & 1 \end{bmatrix}$$





$\underline{\underline{\sigma}}' = \underline{\underline{a}} \underline{\underline{\sigma}} \underline{\underline{a}}^T$ with $\sigma_{i3} = 0$, gives

$$\sigma_{11}' = \left(\frac{1 + c2\theta}{2}\right) \sigma_{11} + s2\theta \sigma_{12} + \left(\frac{1 - c2\theta}{2}\right) \sigma_{22}$$

$$\sigma_{22}' = \left(\frac{1 - c2\theta}{2}\right) \sigma_{11} - s2\theta \sigma_{12} + \left(\frac{1 + c2\theta}{2}\right) \sigma_{22}$$

$$\sigma_{12}' = \left(\frac{\sigma_{22} - \sigma_{11}}{2}\right) s2\theta + \sigma_{12} c2\theta$$

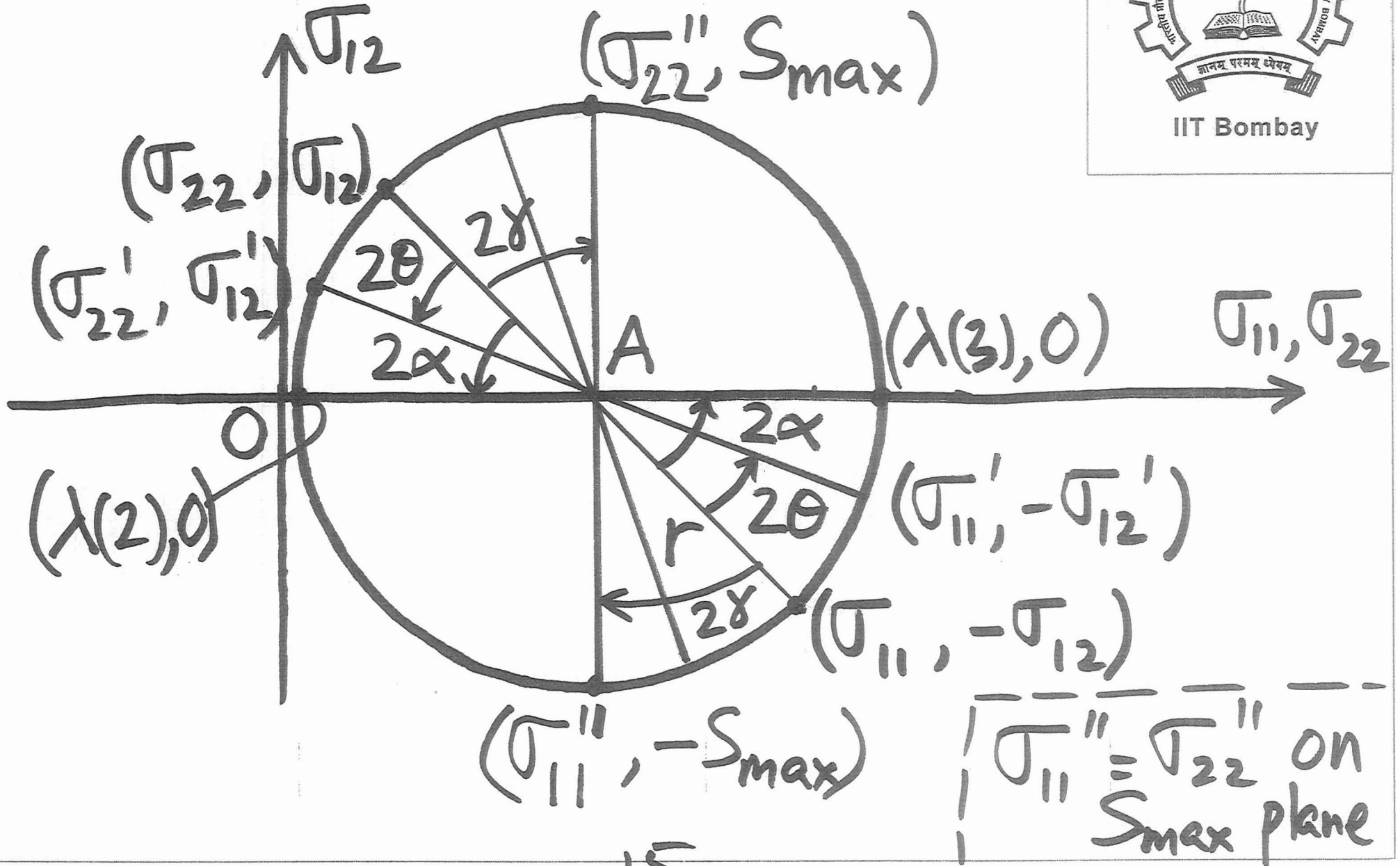
$$\sigma_{i3}' = 0$$

$$\Rightarrow \sigma_{12}' = 0 \text{ for } \tan 2\theta = \left(\frac{2\sigma_{12}}{\sigma_{11} - \sigma_{22}}\right)$$

get $\sigma_{11}' = \lambda(2)$, $\sigma_{22}' = \lambda(3)$
 p.10

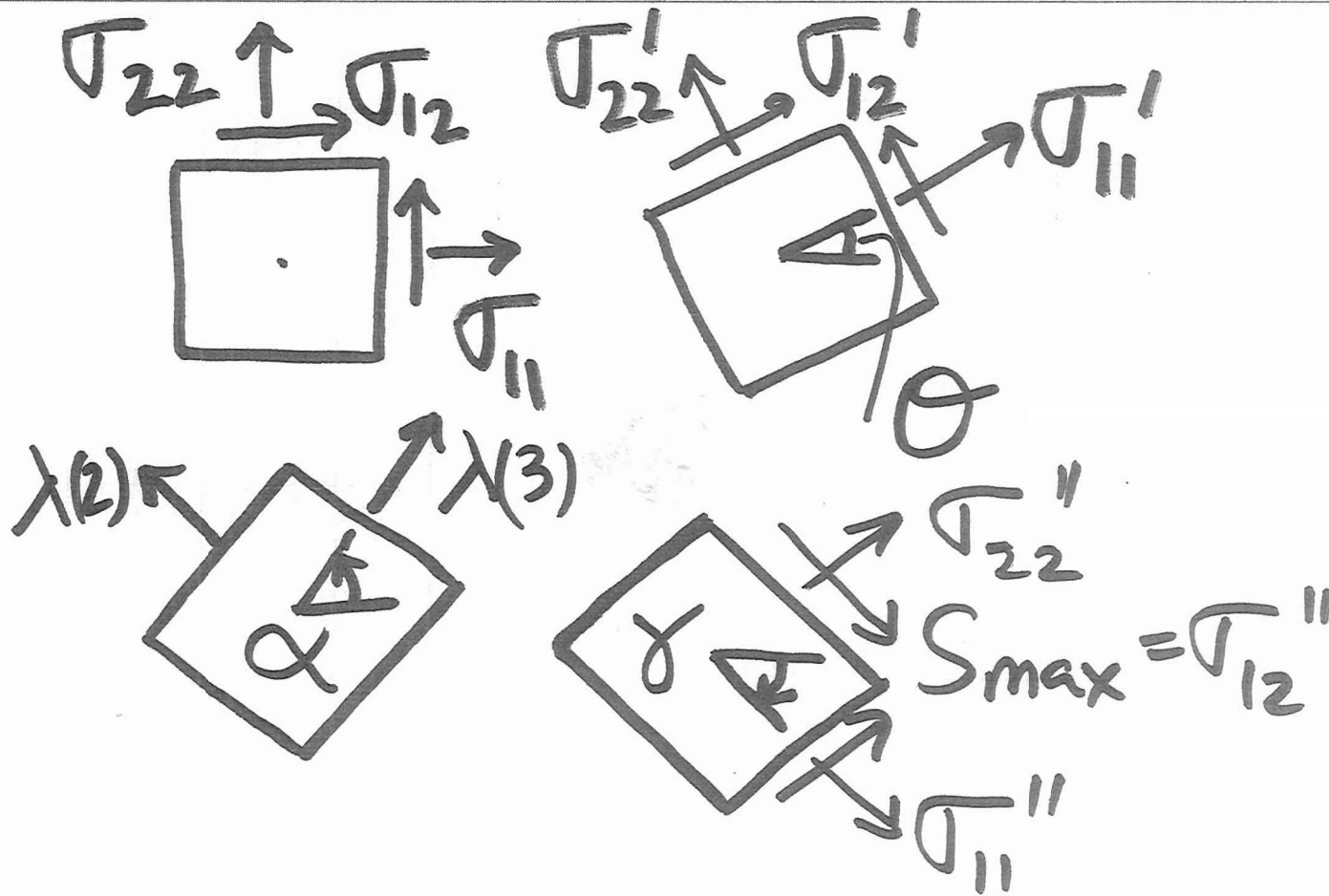
Subst

Mohr's Circle - 2D





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$$OA = \sigma_{22} + \frac{\sigma_{11} - \sigma_{22}}{2} = \frac{\sigma_{11} + \sigma_{22}}{2}$$

$$r^2 = (\sigma_{12})^2 + (\sigma_{11} - OA)^2 = \left(\frac{\sigma_{11} - \sigma_{22}}{2}\right)^2 + \sigma_{12}^2$$

$$\sigma_{11}' = r \cos(2\alpha - 2\theta) + OA$$

$$\sigma_{22}' = OA - r \cos(2\alpha - 2\theta)$$

$$\sigma_{12}' = r \sin(2\alpha - 2\theta)$$

$$\cos 2\alpha = \frac{\sigma_{11}' - \sigma_{22}'}{2r}, \quad \sin 2\alpha = \frac{\sigma_{12}'}{r}$$

Subst $OA, r, \cos 2\alpha, \sin 2\alpha$, get σ_{11}' ,
 σ_{22}' , σ_{12}' as on p.14.

$$\lambda(2) = OA + r \quad \left. \vphantom{\lambda(2)} \right\} \text{ same as p.10.}$$

$$\lambda(3) = OA - r \quad \left. \vphantom{\lambda(3)} \right\} \text{ Mohr's circle verified.}$$



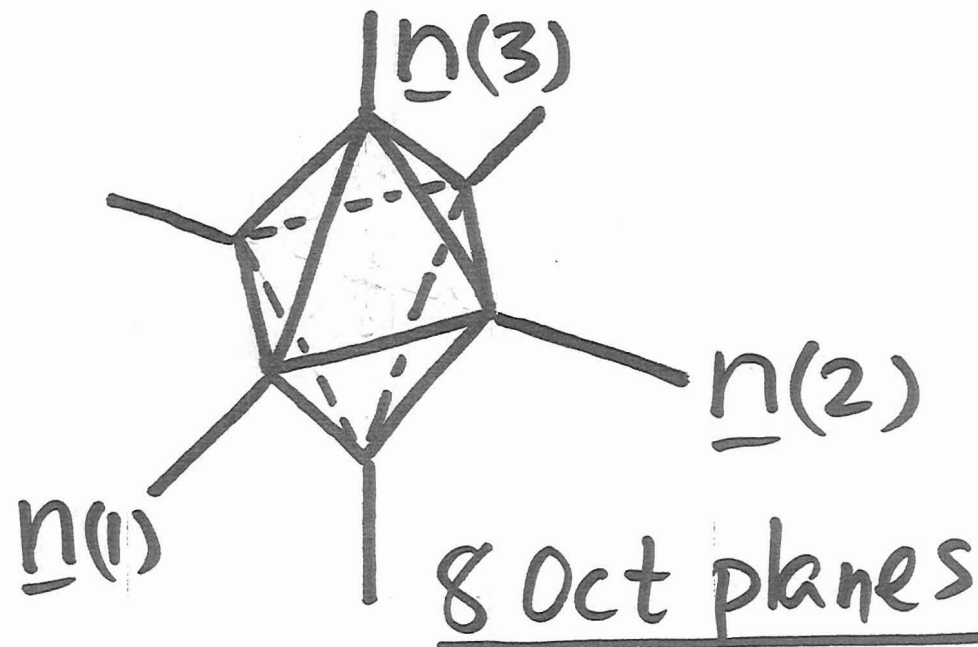
Stresses on Octahedral plane



$$\rightarrow \underline{e}_i = \underline{n}(i)$$

Refer \underline{n} to p-system.

$$\underline{n} = \left(\pm \frac{1}{\sqrt{3}}, \pm \frac{1}{\sqrt{3}}, \pm \frac{1}{\sqrt{3}} \right) \rightarrow \text{Oct. plane.}$$



$$N_{\text{Oct}} = \frac{\lambda(1) + \lambda(2) + \lambda(3)}{3}$$

$$N_{\text{Oct}} = I_1/3$$

$$\underline{t}_{\text{Oct}} = \left(\frac{\lambda(1)}{\pm\sqrt{3}}, \frac{\lambda(2)}{\pm\sqrt{3}}, \frac{\lambda(3)}{\pm\sqrt{3}} \right)$$

$$S_{oct}^2 = (\lambda^2(1) + \lambda^2(2) + \lambda^2(3)) / 3 - (I_1/3)^2$$

$$I_1^2 = \lambda^2(1) + \lambda^2(2) + \lambda^2(3) + 2(\lambda(1)\lambda(2) + \lambda(2)\lambda(3) + \lambda(3)\lambda(1))$$

$$I_2 = \lambda(1)\lambda(2) + \lambda(2)\lambda(3) + \lambda(3)\lambda(1)$$

$$S_{oct}^2 = \frac{(I_1^2 - 2I_2)}{3} - (I_1/3)^2 = \frac{2}{9} I_1^2 - \frac{2}{3} I_2$$



LECTURE 6

STRESS ANALYSIS.



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- Pure shear state of stress
- Deviatoric stress
- Max shear stress.
- Equilibrium equations.

Pure Shear State of Stress.



L6

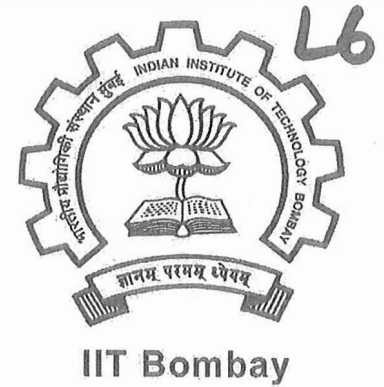
If $\underline{\underline{\sigma}} \rightarrow \underline{\underline{\sigma}}'$:

$\underline{\underline{\sigma}}' = \begin{bmatrix} 0 & \sigma'_{12} & \sigma'_{13} \\ \text{Symm} & 0 & \sigma'_{23} \\ & & 0 \end{bmatrix}$, then $\underline{\underline{\sigma}}$ is pure shear state of stress.

Necessary & sufficient condit for pure shear state to exist is

$$\sigma_{ii} = 0 = \text{trace}(\underline{\underline{\sigma}}) = I_1$$

Necessary : $\sigma_{11}' + \sigma_{22}' + \sigma_{33}' = I_1$
 $= 0 = \sigma_{xx} + \sigma_{yy} + \sigma_{zz}$



Sufficient :

$I_1 = 0 \Rightarrow$ at least one diagonal comp > 0 & one < 0 .

Say $\sigma_{11} > 0, \sigma_{22} < 0$.

Do rot in 1-2 plane $\rightarrow \underline{\underline{a}} = \begin{bmatrix} c\theta & s\theta & 0 \\ -s\theta & c\theta & 0 \\ 0 & 0 & 1 \end{bmatrix}$
 so that $\sigma_{11}'' = 0$. Possible?

$$\sigma_{33}'' = a_{3r} a_{3s} \sigma_{rs} = \sigma_{33}$$

$$\sigma_{11}'' = a_{1r} a_{1s} \sigma_{rs} = c^2 \theta \sigma_{11} + s^2 \theta \sigma_{22} + 2c\theta s \theta \sigma_{12}$$

||?

$$0 = x \sigma_{11} + (1-x) \sigma_{22} \pm 2\sqrt{x} \sqrt{1-x} \sigma_{12},$$

$$(\sigma_{11}^2 + \sigma_{22}^2 + 4\sigma_{12}^2 - 2\sigma_{11}\sigma_{22})x^2 + (2\sigma_{11}\sigma_{22} - 2\sigma_{22}^2 - 4\sigma_{12}^2)x + \sigma_{22}^2 = 0$$

$x = \cos^2 \theta$
A B C

$$x = \frac{\sqrt{2(\sigma_{22}^2 + 2\sigma_{12}^2 - \sigma_{11}\sigma_{22})} \pm 4\sqrt{\sigma_{12}^4 - \sigma_{11}\sigma_{22}\sigma_{12}}}{2A}$$



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$$\sigma_{22} < 0 \Rightarrow A > 0, -B > 0, C > 0$$

$$\Rightarrow -B > \sqrt{B^2 - 4AC}$$

$$\Rightarrow x > 0, \cos\theta \text{ real,}$$

transformation: $\sigma_{11}'' = 0$ possible.

$$\Rightarrow \sigma_{11}'' = 0, \sigma_{22}'' \text{ \& \} \sigma_{33}'' (= \sigma_{33}) \text{ have}$$

opp. sign $\therefore I_1 = 0$. Do 2-3 rotation

$\therefore \sigma_{22}' = 0$ using same arguments.

$\therefore I_1 = 0, \sigma_{33}' = 0$ simultaneously.

[Note $\sigma_{11}' = \sigma_{11}'' = 0$ (\therefore 2-3 rot)] QED



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Deviatoric Stress.



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$$\sigma_{ij} = \hat{\sigma}_{ij} + \frac{1}{3} \sigma_{kk} \delta_{ij}$$

Deviatoric - Cubical - state of stress

$$\underline{\underline{\sigma}} = \underline{\underline{\hat{\sigma}}} + \begin{bmatrix} I_1/3 & 0 & 0 \\ 0 & I_1/3 & 0 \\ 0 & 0 & I_1/3 \end{bmatrix} \text{ --- pure hydrostatic state}$$

Contraction $\rightarrow \sigma_{ii} = \hat{\sigma}_{ii} + \frac{1}{3} \sigma_{kk} \delta_{ii}$
 $\hat{\sigma}_{ii} = 0$, $\underline{\underline{\hat{\sigma}}}$ pure shear state

Maximum Shear Stress



So far

- Principal Stresses / Max-min

Normal Stress, $t = \lambda \underline{n}$, $\sigma'_{ij} = 0$ if $j \neq i$,
(on \underline{e}'_i planes)

N extremum, $S = 0$. Transf always exists

- Pure Shear State, $\sigma'_{ij} = 0$, $i = j$,
in general

on \underline{e}'_i planes $N = 0$, $S \neq$ extremum, λ ,

Transf. exists iff $I_1 = 0$.

Now seek \underline{e}_i planes on which $S = \text{extremum}$, $N \neq 0$ in general.

Refer to p-axes system (\underline{e}_i)

$$S^2 = (\lambda(1)n_1)^2 + (\lambda(2)n_2)^2 + (\lambda(3)n_3)^2$$

$$- [\lambda(1)n_1^2 + \lambda(2)n_2^2 + \lambda(3)n_3^2]^2$$

(L5, p.5)

$$\frac{\partial S^2}{\partial n_1} = \frac{\partial S^2}{\partial n_2} = \frac{\partial S^2}{\partial n_3} = 0 \quad \text{won't work } \because \underline{|n| = 1} \text{ constraint.}$$

Can eliminate, say, $n_3^2 = 1 - n_1^2 - n_2^2$, then



$\partial S^2 / \partial n_1 = \partial S^2 / \partial n_2 = 0$ will work
— tedious.

So use Lagrange Multiplier
approach.

$$G = S^2 + L(1 - n_1^2 - n_2^2 - n_3^2)$$

ie $G = S^2$ if constraint satisfied.

Now L (Lagr Mult), n_1, n_2, n_3 are
indep \therefore constraint built in.



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$$\frac{\partial G}{\partial L} = 1 - n_1^2 - n_2^2 - n_3^2 = 0 \rightarrow (a)$$

$$\frac{\partial G}{\partial n_1} = n_1 \left\{ \lambda^2(1) - 2[\lambda(1)n_1^2 + \lambda(2)n_2^2 + \lambda(3)n_3^2] \lambda(1) + L \right\} = 0 \rightarrow (b)$$

$$\frac{\partial G}{\partial n_2} = n_2 \left\{ \lambda^2(2) - 2[\lambda(1)n_1^2 + \lambda(2)n_2^2 + \lambda(3)n_3^2] \lambda(2) + L \right\} = 0 \rightarrow (c)$$

$$\frac{\partial G}{\partial n_3} = n_3 \left\{ \lambda^2(3) - 2[\lambda(1)n_1^2 + \lambda(2)n_2^2 + \lambda(3)n_3^2] \lambda(3) + L \right\} = 0 \rightarrow (d)$$



Since (a) represents constraint, stationary values of G same as those of S^2 , ie treating λ, n_1, n_2, n_3 as independent works!!



Case (i): ^{only} One comp of \underline{n} non-zero. ^{same as} $S^2 = 0$

$n_1=1, n_2=n_3=0, L=\lambda^2(1), S^2=0$	} P-stress soln, S is stationary but not max.
$n_2=1, n_1=n_3=0, L=\lambda^2(2), S^2=0$	
$n_3=1, n_1=n_2=0, L=\lambda^2(3), S^2=0$	

Case (ii): one comp of \underline{n} zero,
two non-zero.

$$n_1 = 0, n_2 \neq 0, n_3 \neq 0$$

$$(a), (c), (d), \rightarrow \lambda^2(2) - 2[\lambda(2)n_2^2 + \lambda(3) - \lambda(3)n_2^2]\lambda(2)$$

$$+ L = 0$$

$$\lambda^2(3) - 2[\lambda(2)n_2^2 + \lambda(3) - \lambda(3)n_2^2]\lambda(3) + L = 0$$

$$\Rightarrow [\lambda(2) - \lambda(3)][\lambda(2) + \lambda(3) - 2(\lambda(2)n_2^2 + \lambda(3) - \lambda(3)n_2^2)]$$

Either $\lambda(2) = \lambda(3)$, $n_2 = \text{arb}$, $L = \lambda^2(2)$, $S_2 = 0$

or $\lambda(2) \neq \lambda(3)$, $n_2 = \pm \frac{1}{\sqrt{2}}$, $n_3 = \pm \frac{1}{\sqrt{2}}$, $L = \lambda(2)\lambda(3)$, $S_2 = 0$



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L6

$$[\lambda(2) - \lambda(3)][1 - 2n_2^2] = 0$$

|| a

$$S^2 = (\lambda^2(2) + \lambda^2(3) - 2\lambda(2)\lambda(3))/4$$

$$S = \frac{|\lambda(2) - \lambda(3)|}{2}$$



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Similarly for $n_2 = 0, n_1 \neq 0, n_3 \neq 0$
 & for $n_3 = 0, n_1 \neq 0, n_2 \neq 0$

Summary case (ii):

$$n_1 = 0, n_2 = \pm \frac{1}{\sqrt{2}}, n_3 = \pm \frac{1}{\sqrt{2}}; \underbrace{n_2 = \text{arb}, n_3 = \pm \sqrt{1 - n_2^2}}_{\lambda(2) = \lambda(3)}$$

$$L = \lambda(2)\lambda(3), S = \frac{1}{2} |\lambda(2) - \lambda(3)|$$

$$n_2 = 0, n_1 = \pm \frac{1}{\sqrt{2}}, n_3 = \pm \frac{1}{\sqrt{2}} ;$$

$$\lambda(1) \neq \lambda(3)$$

$$n_1 = a + b, n_3 = \pm \sqrt{1 - n_1^2}$$

$$\lambda(1) = \lambda(3)$$

$$L = \lambda(1)\lambda(3),$$

$$S = \frac{1}{2} |\lambda(1) - \lambda(3)|$$

$$n_3 = 0, n_1 = \pm \frac{1}{\sqrt{2}}, n_2 = \pm \frac{1}{\sqrt{2}} ; n_1 = a + b, n_2 = \pm \sqrt{1 - n_1^2}$$

$$\lambda(1) \neq \lambda(2)$$

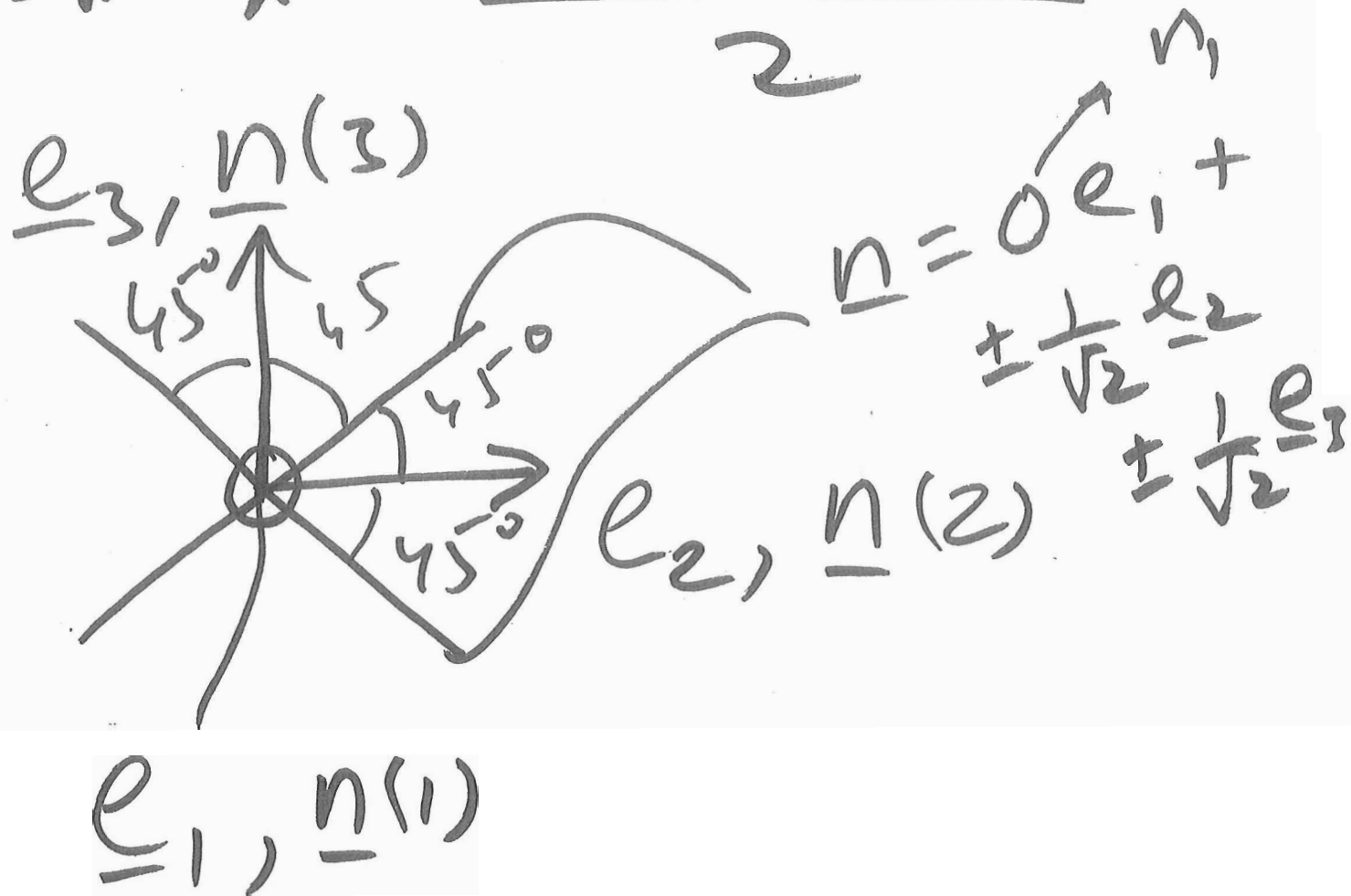
$$\lambda(1) = \lambda(2)$$

$$L = \lambda(1)\lambda(2), S = \frac{1}{2} |\lambda(1) - \lambda(2)|$$



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If
$$S_{max} = \frac{(\lambda(2) - \lambda(3))}{2}$$



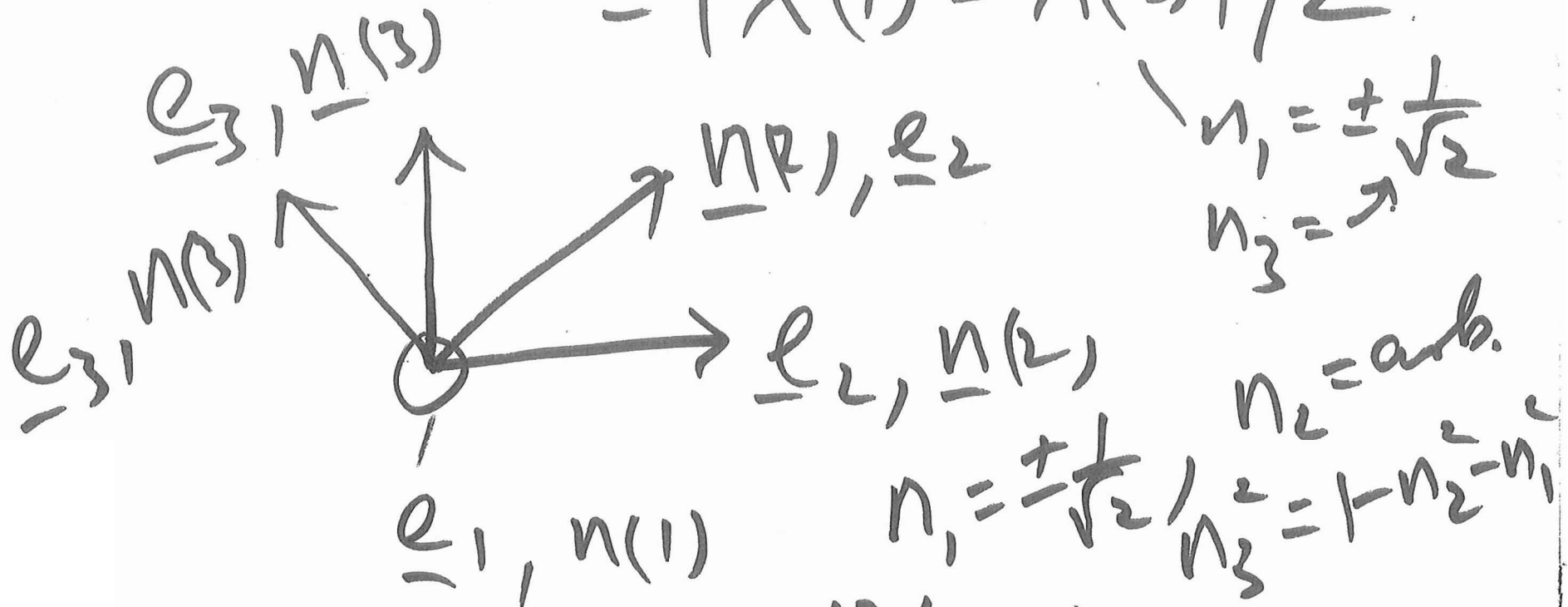
If $\lambda(2) = \lambda(3)$.

$$S_{\max} = \frac{|\lambda(1) - \lambda(2)|}{2}$$

$$n_1 = \pm \frac{1}{\sqrt{2}}$$

$$n_2 = \pm \frac{1}{\sqrt{2}}$$

$$= \frac{|\lambda(1) - \lambda(3)|}{2}$$



$$\# S_{\max} = \max\left(\frac{1}{2}|\lambda(1) - \lambda(2)|, \frac{1}{2}|\lambda(2) - \lambda(3)|, \frac{1}{2}|\lambda(1) - \lambda(3)|\right)$$



S_{\max} occurs on planes containing one p-axis & equally ($\pm 45^\circ$) inclined to other two p-axes, when (all) p-stresses are distinct.

If any two p-stresses same, say $\lambda(1) = \lambda(2)$, $S_{\max} = \frac{1}{2}|\lambda(1) - \lambda(3)| = \frac{1}{2}|\lambda(2) - \lambda(3)|$, acts on plane with $n_3 = \pm \frac{1}{\sqrt{2}}$, $n_1 = \text{arb}$, $n_2 = \pm \sqrt{1 - n_1^2 - n_3^2}$

$\therefore \underline{n(1)}, \underline{n(2)}$ arbitrary but
 \perp to each other & to $\underline{n(3)}$.

Ref 2D Mohr's circle (L5, p.15)
to see results recovered for 2D case.

Case (iii): $n_1 \neq 0, n_2 \neq 0, n_3 \neq 0$.

Here (b, c, d), p.9, give inconsistent
system of eqns in n_1^2, n_2^2, n_3^2 , unless
 $\lambda(1) = \lambda(2) = \lambda(3)$. See $\det(\text{coeff mat}) = 0$

So only possible soln^{ie} for equal p-stresses,
yields $S = 0$ on all planes (as before)



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Equilibrium Equations.



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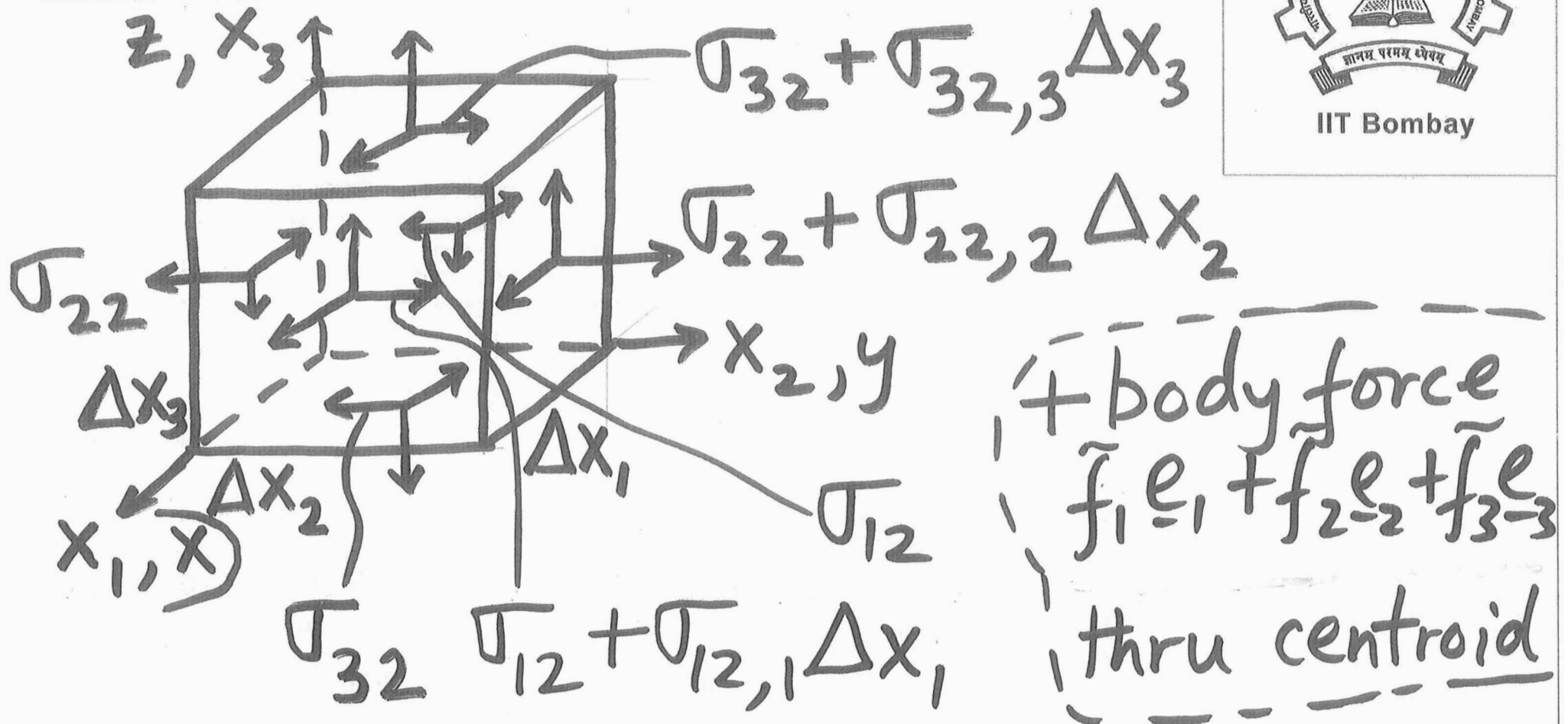


Fig: Stresses on infinitesimal parallelepiped element at P.

$$\Sigma F_y = 0 :$$

$$\begin{aligned}
 & (\cancel{\sigma_{22}} + \sigma_{22,2} \Delta x_2 - \cancel{\sigma_{22}}) \Delta x_1 \Delta x_3 \\
 & + (\cancel{\sigma_{12}} + \sigma_{12,1} \Delta x_1 - \cancel{\sigma_{12}}) \Delta x_2 \Delta x_3 \\
 & + (\cancel{\sigma_{32}} + \sigma_{32,3} \Delta x_3 - \cancel{\sigma_{32}}) \Delta x_1 \Delta x_2 \\
 & + f_2 \Delta x_1 \Delta x_2 \Delta x_3 = 0
 \end{aligned}$$

$$\sigma_{12,1} + \sigma_{22,2} + \sigma_{32,3} + \tilde{f}_2 = 0$$

Similarly for $\Sigma F_x = 0, \Sigma F_z = 0.$

$\sigma_{ji,j} + \tilde{f}_i = 0$

}
EQUIL EQ
(CARTESIAN)



46

Lecture 7

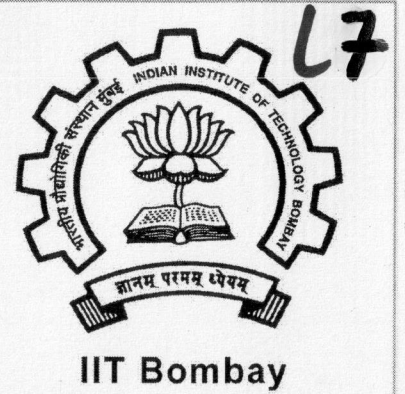
STRESS ANALYSIS.



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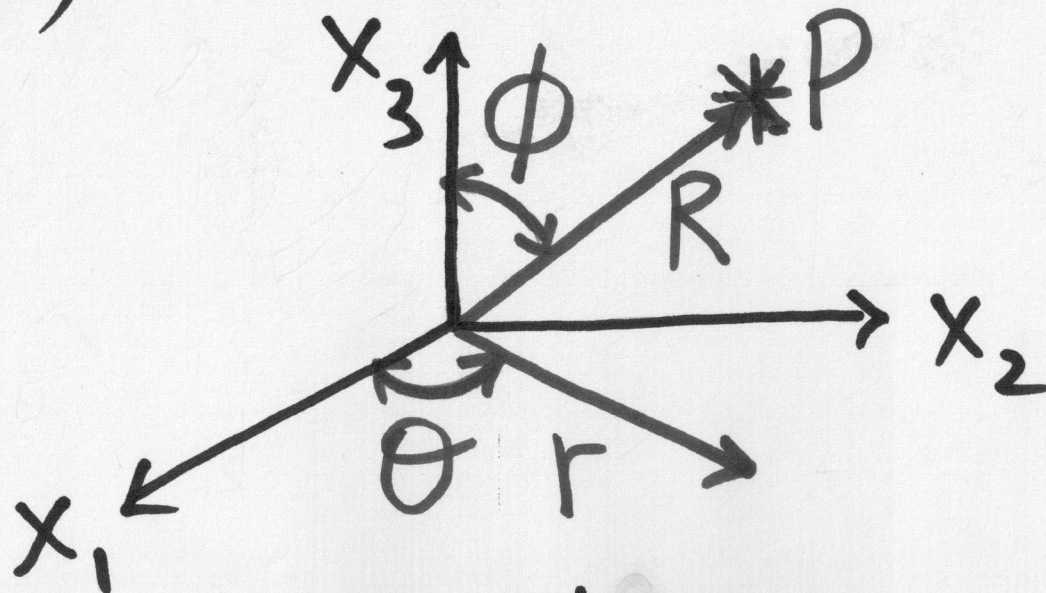
- Equilibrium in Curvilinear Coordinates
- Boundary conditions (stress)
- Problems.

Equilibrium equations in curvilinear orthogonal coordinates



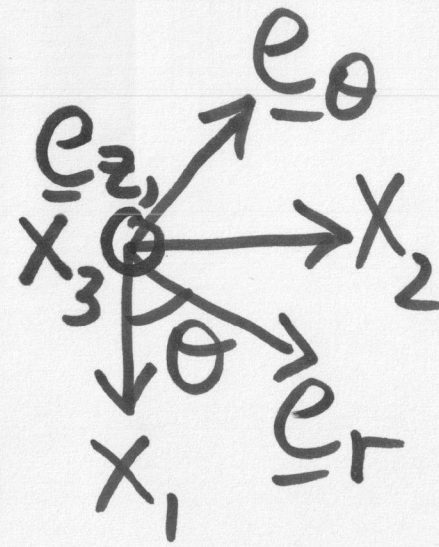
$(r, \theta, z) \rightarrow$ cylindrical coords.

$(R, \theta, \phi) \rightarrow$ spherical coords.



for cyl.

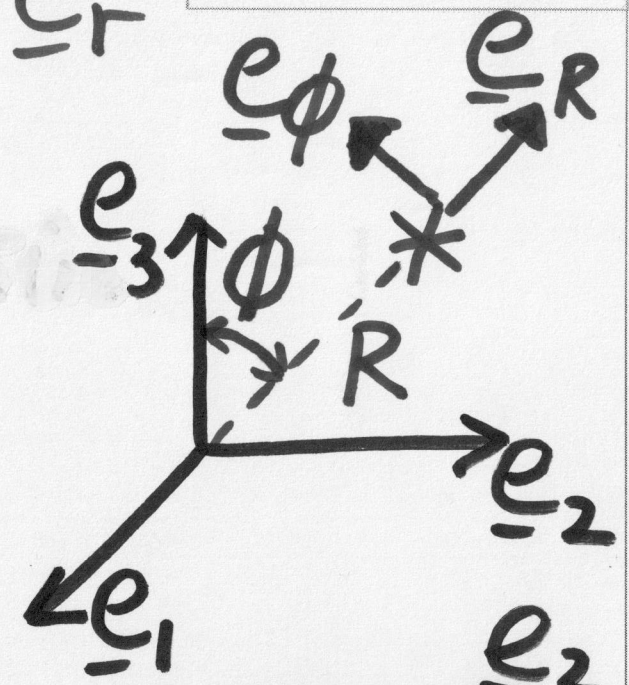
$$\underline{\underline{a}} = \begin{bmatrix} c\theta & s\theta & 0 \\ -s\theta & c\theta & 0 \\ 0 & 0 & 1 \end{bmatrix}$$



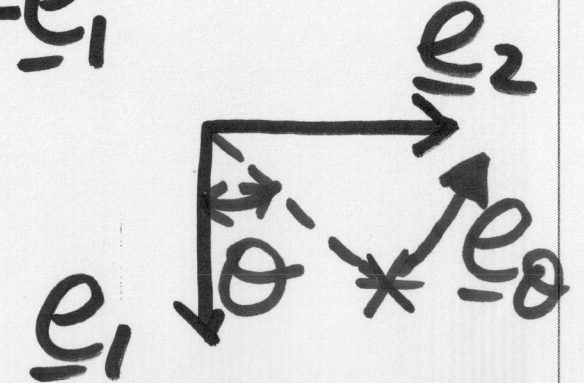
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for spherical

$$\underline{\underline{a}} = \begin{bmatrix} s\phi c\theta & s\phi s\theta & c\phi \\ -s\theta & c\theta & 0 \\ -c\phi c\theta & -c\phi s\theta & s\phi \end{bmatrix}$$



so $\underline{e}_R \times \underline{e}_\theta = \underline{e}_\phi$





L7

$$\sigma_{ji} = a_{rj} a_{si} \sigma'_{rs}$$

$$\frac{\partial(\quad)}{\partial x_j} = \frac{\partial(\quad)}{\partial x'_k} \frac{\partial x'_k}{\partial x_j}$$

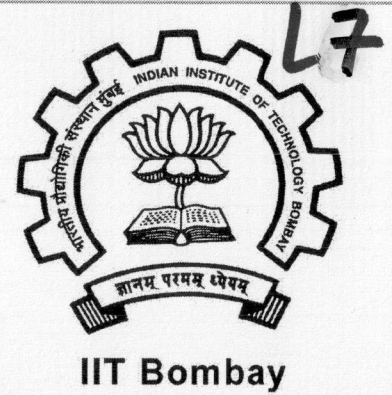
transf between
 a_{kj} for RCC systems
 else $\frac{\partial r}{\partial x}$ etc

Will give
 transf equil eqns

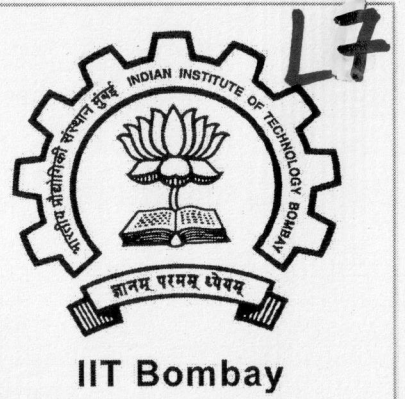
$$0 = \sigma_{ji,j} + \tilde{f}_i = (a_{rj} a_{si} \sigma'_{rs})_{,k} \frac{\partial x'_k}{\partial x_j}$$

eg $\sigma'_{rs} \equiv \begin{bmatrix} \sigma_{rr} & \sigma_{r\theta} & \sigma_{rz} \\ \sigma_{\theta r} & \sigma_{\theta\theta} & \sigma_{\theta z} \\ \sigma_{zr} & \sigma_{z\theta} & \sigma_{zz} \end{bmatrix}, (\quad)_{,k} = \begin{Bmatrix} (\quad)_{,r} \\ (\quad)_{,\theta} \\ (\quad)_{,z} \end{Bmatrix}$

In general curvilinear
 coordinates (see eg. Boresi
 & Schmidt, Advanced
 Mechanics of Materials, 2003)



$$\begin{aligned}
 & \frac{\partial (\beta \gamma \sigma_{xx})}{\partial x} + \frac{\partial (\gamma \alpha \tau_{xy})}{\partial y} + \frac{\partial (\alpha \beta \tau_{xz})}{\partial z} \\
 & + \gamma \tau_{xy} \frac{\partial \alpha}{\partial y} + \beta \tau_{xz} \frac{\partial \alpha}{\partial z} - \gamma \tau_{yy} \frac{\partial \beta}{\partial x} \\
 & - \beta \tau_{zz} \frac{\partial \gamma}{\partial x} + \alpha \beta \gamma \tilde{f}_x = 0
 \end{aligned}$$



$$\begin{aligned}
 & \frac{\partial(\beta\gamma\sigma_{xy})}{\partial x} + \frac{\partial(\gamma\alpha\tau_{yy})}{\partial y} \\
 & + \frac{\partial(\alpha\beta\tau_{yz})}{\partial z} + \gamma\frac{\partial\beta}{\partial x}\tau_{xy} + \alpha\frac{\partial\beta}{\partial z}\tau_{yz} \\
 & - \gamma\frac{\partial\alpha}{\partial y}\sigma_{xx} - \alpha\frac{\partial\gamma}{\partial y}\tau_{zz} + \alpha\beta\gamma\tilde{f}_y = 0 \\
 & \frac{\partial(\beta\gamma\sigma_{xz})}{\partial x} + \frac{\partial(\gamma\alpha\tau_{yz})}{\partial y} + \frac{\partial(\alpha\beta\tau_{zz})}{\partial z} + \beta\frac{\partial\gamma}{\partial x}\sigma_{xz} \\
 & + \alpha\frac{\partial\gamma}{\partial y}\tau_{yz} - \alpha\frac{\partial\beta}{\partial z}\tau_{yy} - \beta\frac{\partial\alpha}{\partial z}\sigma_{xx} + \alpha\beta\gamma\hat{f}_z
 \end{aligned}$$

where

$$ds^2 = \alpha^2 dx^2 + \beta^2 dy^2 + \gamma^2 dz^2$$

= (diagonal of element)



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Cartesian: $\alpha = \beta = \gamma = 1$, $ds^2 = dx^2 + dy^2 + dz^2$

$x \rightarrow x$, $y \rightarrow y$, $z \rightarrow z$

Cylindrical: $ds^2 = dr^2 + r^2 d\theta^2 + dz^2$

$\alpha = 1$, $\beta = r$, $\gamma = 1$, $x \rightarrow r$, $y \rightarrow \theta$, $z \rightarrow z$

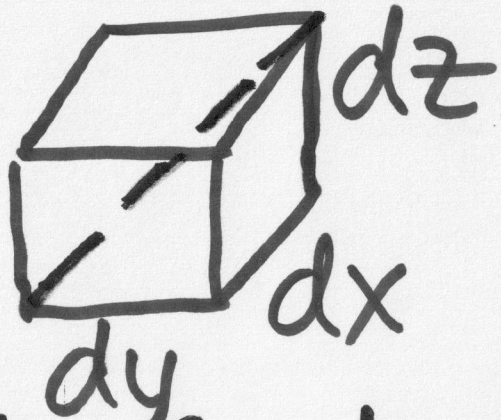
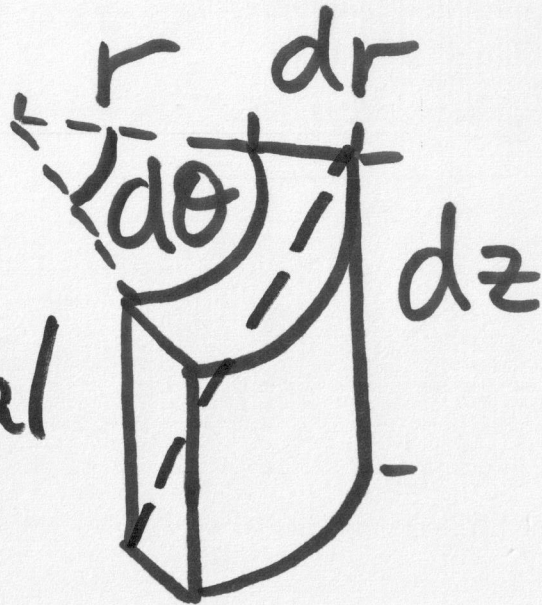
Spherical: $ds^2 = dR^2 + (R \sin \phi)^2 d\theta^2 + R^2 d\phi^2$

$\alpha = 1$, $\beta = R \sin \phi$, $\gamma = R$, $x \rightarrow R$, $y \rightarrow \theta$, $z \rightarrow \phi$



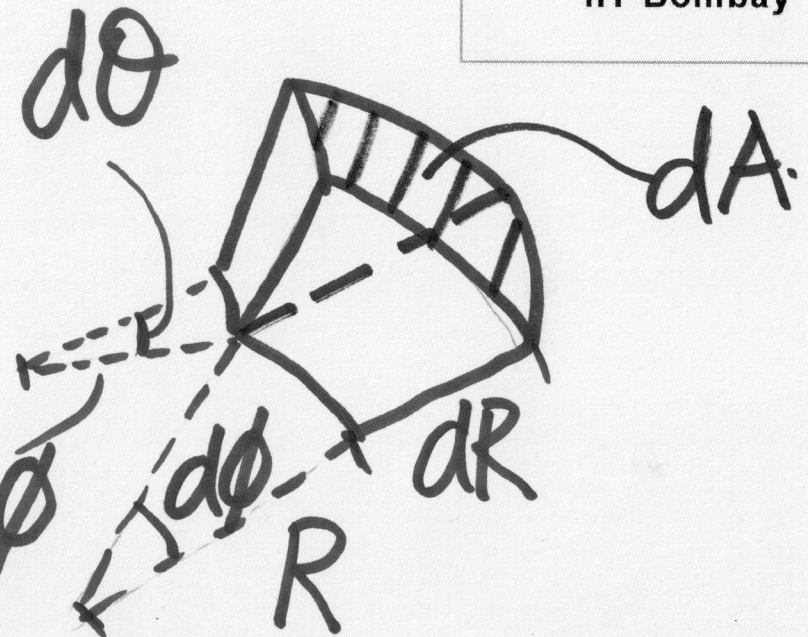
17

Cylindrical element



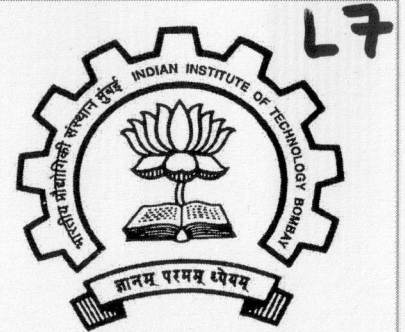
Rect. Cartesian element

6A

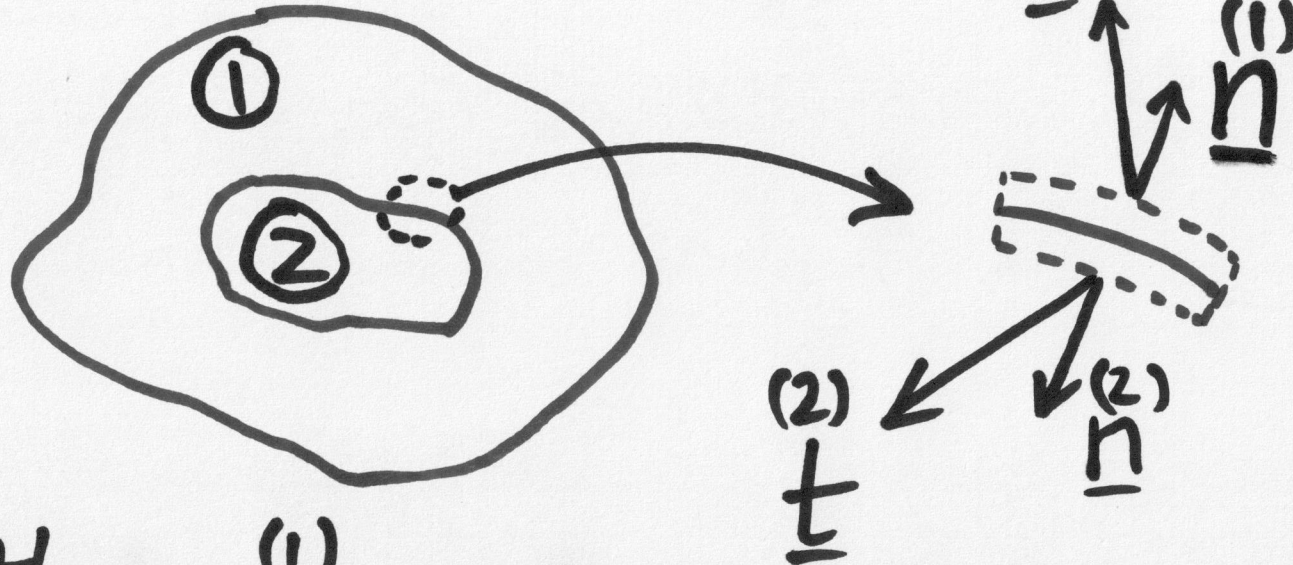


Spherical element

Boundary Conditions.



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Thin slice
around
boundary.

$$\sum \vec{F} = 0 \Rightarrow \left(\vec{t}^{(1)} + \vec{t}^{(2)} \right) dA = 0$$

$$\Rightarrow \sigma^{(1)} \vec{n}^{(1)} + \sigma^{(2)} \vec{n}^{(2)} = 0$$

(neglect
side faces)

$$\left(\underline{\underline{\sigma}}^{(1)} - \underline{\underline{\sigma}}^{(2)} \right) \underline{n} = 0 \quad (\text{ie defines boundary})$$

$\therefore \underline{n}$ not arbitrary,

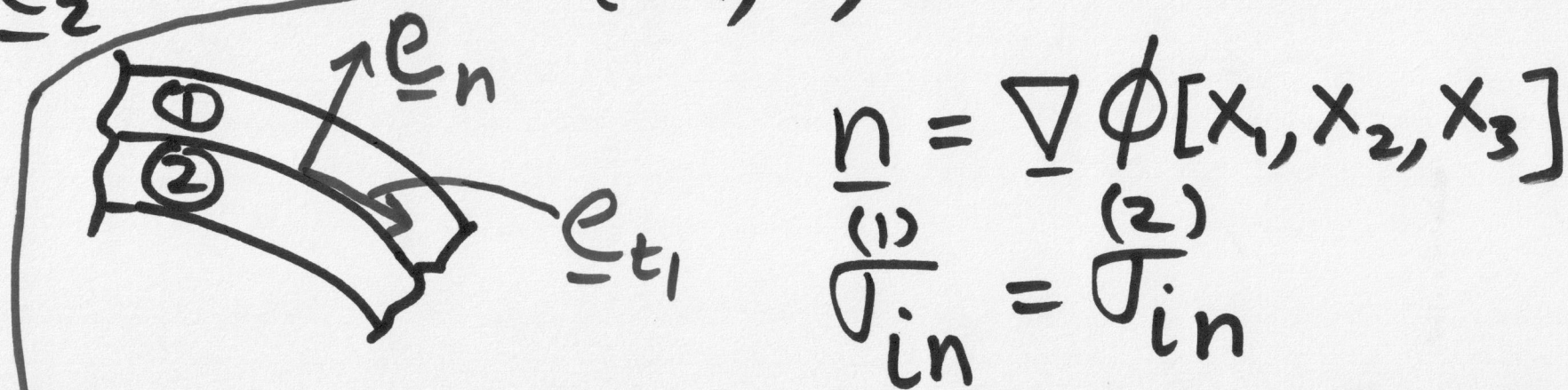
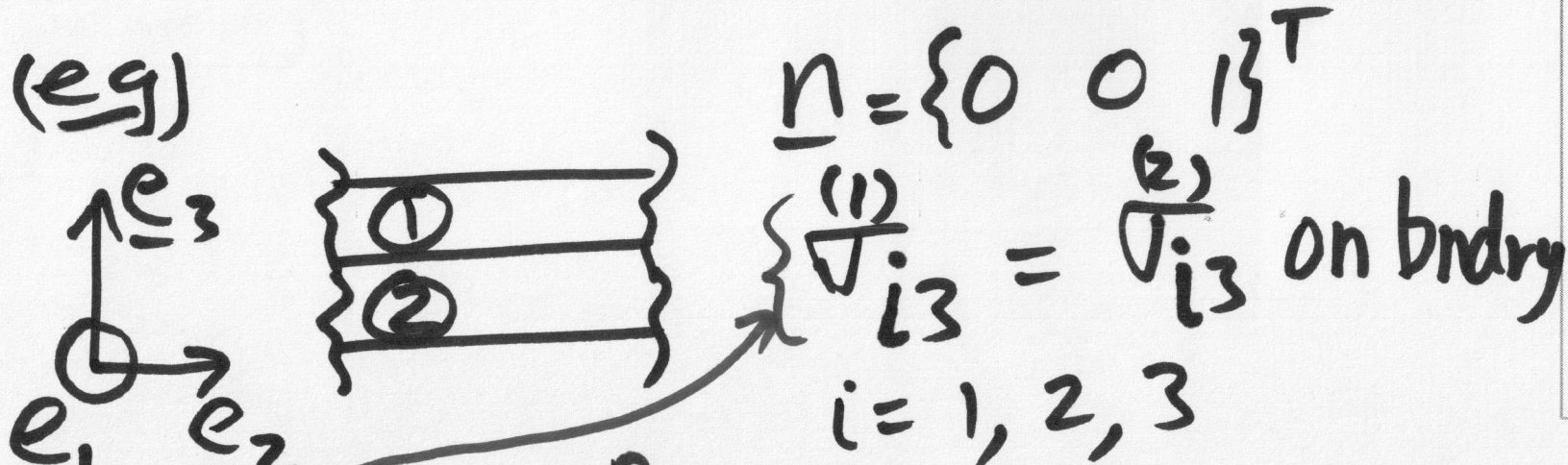
$$\underline{\underline{\sigma}}^{(1)} \underline{n} = \underline{\underline{\sigma}}^{(2)} \underline{n}$$

$$\sigma_{ij}^{(1)} n_j = \sigma_{ij}^{(2)} n_j$$

ie., $\sigma_{11}^{(1)} n_1 + \sigma_{12}^{(1)} n_2 + \sigma_{13}^{(1)} n_3 = \sigma_{11}^{(2)} n_1 + \sigma_{12}^{(2)} n_2 + \sigma_{13}^{(2)} n_3$
 $\sigma_{21}^{(1)} n_1 + \sigma_{22}^{(1)} n_2 + \sigma_{23}^{(1)} n_3 = \sigma_{21}^{(2)} n_1 + \sigma_{22}^{(2)} n_2 + \sigma_{23}^{(2)} n_3$
 $\sigma_{31}^{(1)} n_1 + \sigma_{32}^{(1)} n_2 + \sigma_{33}^{(1)} n_3 = \sigma_{31}^{(2)} n_1 + \sigma_{32}^{(2)} n_2 + \sigma_{33}^{(2)} n_3$



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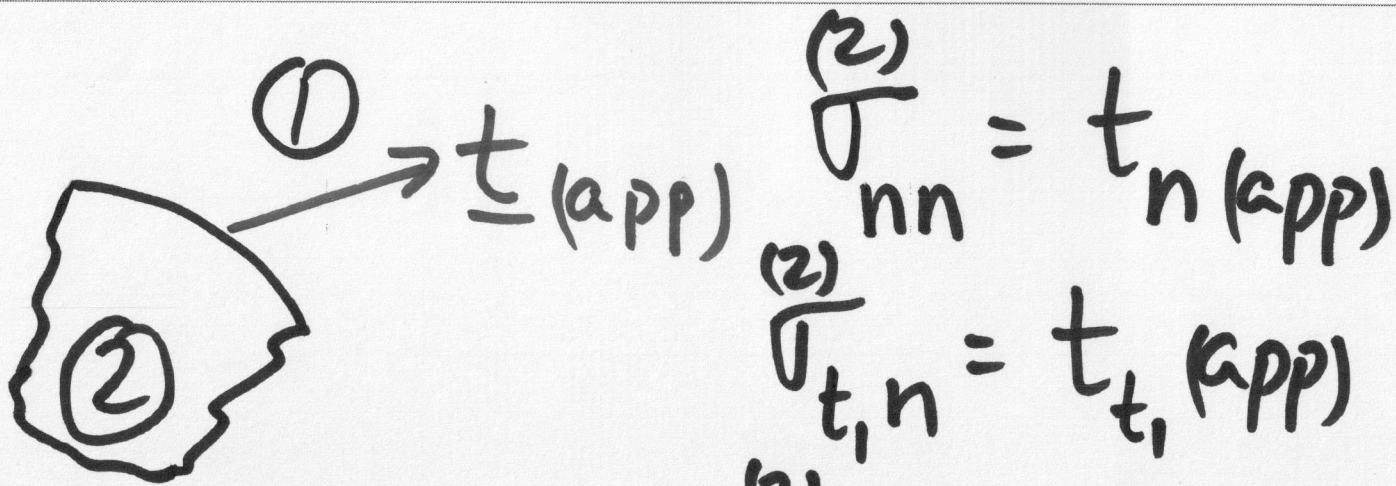


for other stress comps
no continuity across bndry.

ie

$\sigma_{nn}^{(1)} = \sigma_{nn}^{(2)} \rightarrow \text{on bndry}$

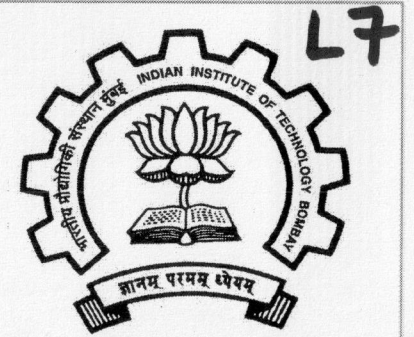
$\sigma_{t_1 n}^{(1)} = \sigma_{t_1 n}^{(2)}, \sigma_{t_2 n} = \sigma_{t_2 n}$



$$\int_{nn}^{(2)} = t_n(app)$$

$$\int_{t_1, n}^{(2)} = t_{t_1}(app)$$

$$\int_{t_2, n}^{(2)} = t_{t_2}(app)$$



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If ① & ② same, then \underline{n} arbitrary

$\Rightarrow \int_{ij}^{(1)} = \int_{ij}^{(2)}$ ie all comps continuous.
(as expected).

P5 At a point in a solid
principal stresses are

$$\lambda(1) = 1, \lambda(2) = 4, \lambda(3) = -2$$

p-axes are $\underline{n}(1) = \left(\frac{1}{2}, \frac{1}{2}, \sqrt{\frac{1}{2}}\right)$

$$\underline{n}(2) = (0, \quad , \quad)$$

Find $\underline{\sigma}$.

$$\underline{n}(1) \cdot \underline{n}(2) = 0 = n_2(2) \cdot \frac{1}{2} + n_3(2) \cdot \frac{1}{\sqrt{2}}$$

$$\underline{n}(2) \cdot \underline{n}(2) = 1 = n_2^2(2) + n_3^2(2)$$



$$\Rightarrow n_2(2) = \mp \sqrt{\frac{2}{3}}, n_3(2) = \pm \sqrt{\frac{1}{3}}$$

$$\underline{n}(3) = \underline{n}(1) \times \underline{n}(2)$$

$$= \left(\sqrt{\frac{1}{12}} + \sqrt{\frac{1}{3}}, \sqrt{\frac{1}{12}}, -\sqrt{\frac{1}{6}} \right)$$

Let x_i' be p-axes system, ie
 $\underline{e}'_1 = \underline{n}(1), \underline{e}'_2 = \underline{n}(2), \underline{e}'_3 = \underline{n}(3)$

$$\underline{a} = \begin{bmatrix} 1/2 & 1/2 & 1/\sqrt{2} \\ 0 & -\sqrt{2/3} & \sqrt{1/3} \\ \sqrt{1/12} + \sqrt{1/3} & -\sqrt{1/12} & -\sqrt{1/6} \end{bmatrix}$$



$$\sigma_{ij} = a_{ri} a_{sj} \sigma_{rs} / \sigma = \underline{a}^T \underline{\sigma} \underline{a}$$

$$\underline{\sigma}' = \begin{bmatrix} \lambda(1) & 0 & 0 \\ 0 & \lambda(2) & 0 \\ 0 & 0 & \lambda(3) \end{bmatrix} = \begin{bmatrix} 1 & 0 & 0 \\ 0 & 4 & 0 \\ 0 & 0 & -2 \end{bmatrix}$$

$$\underline{\sigma} = \begin{bmatrix} -1.25 & 0.75 & 1.061 \\ 0.75 & 2.75 & -1.768 \\ 1.061 & -1.768 & 1.5 \end{bmatrix}$$



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L7

P.6. Given $\underline{\underline{\sigma}}$ at a point

$$\underline{\underline{\sigma}} = \begin{bmatrix} 1 & -3 & \sqrt{2} \\ -3 & 1 & -\sqrt{2} \\ \sqrt{2} & -\sqrt{2} & 4 \end{bmatrix}$$

Find: (a) Principal Deviator stresses
& corresponding planes

(b) N_{oct} , S_{oct}

(c) S_{max} & corresponding plane



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Principal stress problem
is

$$(\underline{\underline{\sigma}} - \lambda \underline{\underline{I}}) \underline{n} = \underline{0}$$

Also, $\underline{\underline{\hat{\sigma}}}$ defined as (L6, p.5)

$$\underline{\underline{\sigma}} = \underline{\underline{\hat{\sigma}}} + \frac{I_1}{3} \underline{\underline{I}}$$

$$\Rightarrow (\underline{\underline{\hat{\sigma}}} - (\lambda - \frac{I_1}{3}) \underline{\underline{I}}) \underline{n} = \underline{0}$$

$\hat{\lambda} = \lambda - \frac{I_1}{3} = p$ -deviator str, p -axes remain same.



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Solving evp for λ, \underline{n} , (MATLAB)

$$\lambda(1) = -2, \lambda(2) = 2, \lambda(3) = 6$$

$$\underline{n}(1) = (1/\sqrt{2}, 1/\sqrt{2}, 0)^T$$

$$\underline{n}(2) = (-0.5, 0.5, 1/\sqrt{2})^T$$

$$\underline{n}(3) = (0.5, -0.5, 1/\sqrt{2})^T$$

$$\hat{\lambda}(1) = -2 - \frac{I_1}{3} = -2 - \frac{6}{3} = -4, \hat{\lambda}(2) = 0$$

$$\hat{\lambda}(3) = 6 - 2 = 4$$

$$N_{\text{oct}} = I_1/3 = 2 \blacktriangleleft (L5, p. 18)$$



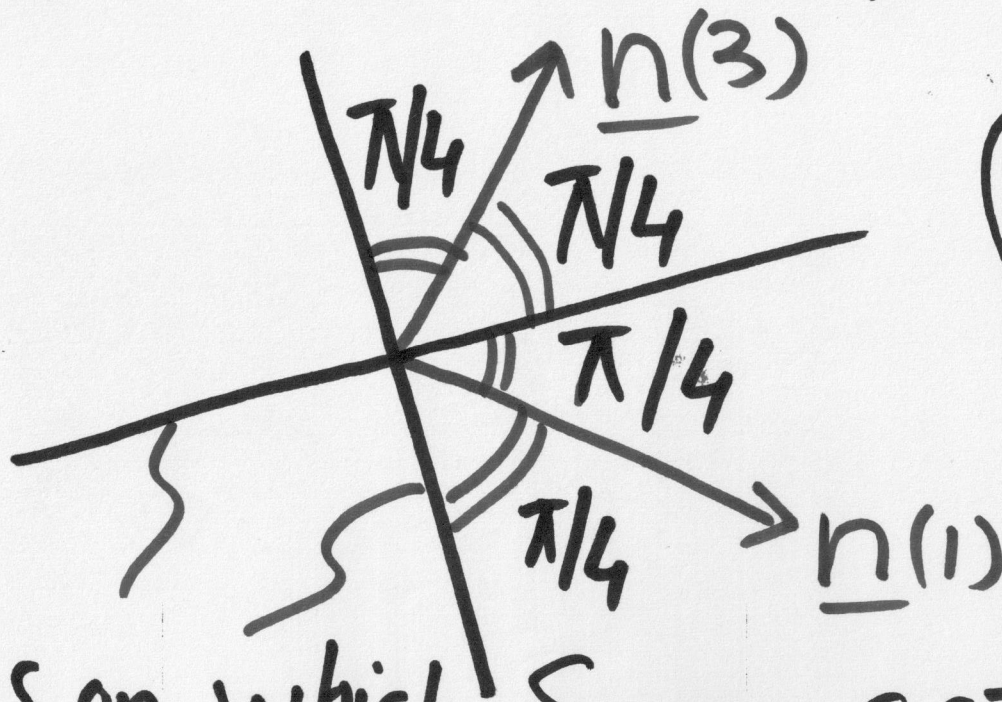
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$$\lambda^3 - 6\lambda^2 - 4\lambda + 24 = 0$$

$$S_{oct}^2 = \frac{2}{9} I_1^2 - \frac{2}{3} I_2 = \frac{2}{9} \cdot 6^2 - \frac{2}{3}(-4)$$

$$S_{oct} = \sqrt{32/3}$$

$$S_{max} = (6 - (-2))/2 = 4$$



planes on which S_{max} acts. $\left\{ \begin{array}{l} \underline{n} \cdot \underline{n}(1) = 1/\sqrt{2} \\ \underline{n} \cdot \underline{n}(3) = \pm 1/\sqrt{2} \\ \underline{n} \cdot \underline{n} = 1 \end{array} \right. \rightarrow \text{solve}$



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L7

P7 Rectangular plate,
thickness $t=1\text{cm}$, lying in
region $0 \leq x_1 \leq 2b$, $-c \leq x_2 \leq c$,
loaded in a manner that yields

$$\sigma_{11} = \frac{q}{2I} \left(x_1^2 x_2 - \frac{2}{3} x_2^3 + \frac{2}{5} c^2 x_2 \right),$$

$$\sigma_{22} = \frac{q}{2I} \left(\frac{1}{3} x_2^3 - c^2 x_2 + \frac{2}{3} c^3 \right), \quad \sigma_{i3} = 0$$

where $I = 2c^3/3$, q is constant.

Assume $\tilde{f}_b = \tilde{m}_b = 0$



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Find : (a) σ_{12} to ensure equil.
 (b) BC's for which soln
 of σ valid.



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Equilibrium eqns yield

$$\sigma_{11,1} + \sigma_{12,2} = 0 = \frac{q}{2I} 2x_1 x_2 + \sigma_{12,2}$$

$$\sigma_{12,1} + \sigma_{22,2} = 0 = \sigma_{12,1} + \frac{q}{2I} (x_2^2 - C^2)$$

$$\Rightarrow \sigma_{12} = \frac{-q}{2I} x_1 x_2^2 + f(x_1) + C_1$$

$$= \frac{-q}{2I} (x_2^2 - C^2) x_1 + g(x_2) + C_2$$

$$\Rightarrow f(x_1) = \frac{q}{2I} c^2 x_1, \quad g(x_2) = 0$$

$$C_1 = C_2 = k.$$

$$\sigma_{12} = -\frac{q}{2I} (x_2^2 - c^2) x_1 + k.$$

$$\text{BC's: } x_1 = 0, \quad (\sigma_{11})_{\text{appl}} = \frac{3q}{2c^3} \left(-\frac{x_2^3}{3} + \frac{c^2}{5} x_2 \right)$$

$$(\sigma_{12})_{\text{appl}} = k$$

$$x_1 = 2b, \quad (\sigma_{11})_{\text{appl}} = \frac{3q}{2c^3} \left(2b^2 x_2 - \frac{x_2^3}{3} + \frac{c^2}{5} x_2 \right)$$

$$(\sigma_{12})_{\text{appl}} = -\frac{3q}{2c^3} (x_2^2 - c^2) b + k$$

-20.



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$$X_2 = C, (\sigma_{22})_{\text{appl}} = \frac{3q}{4C^3} \left(\frac{C^3}{3} - C^3 + \frac{2}{3}C^3 \right) = 0$$

$$X_2 = \pm C, (\sigma_{12})_{\text{appl}} = -\frac{3q}{4C^3} (C^2 - C^2) X_1 + R = R$$

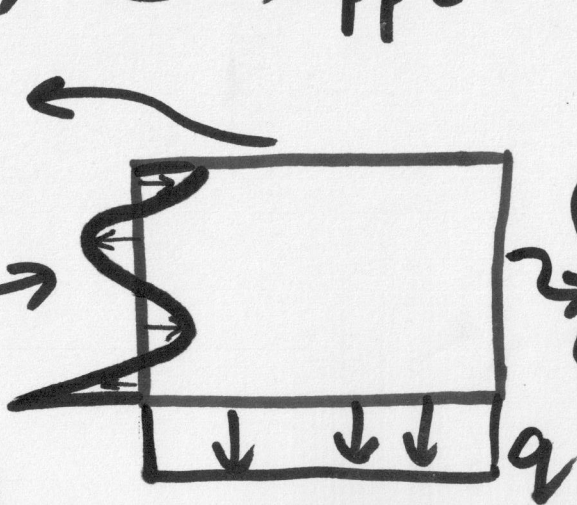
$$X_2 = -C, (\sigma_{22})_{\text{appl}} = \frac{3q}{4C^3} \left(\frac{4}{3}C^3 \right) = q$$



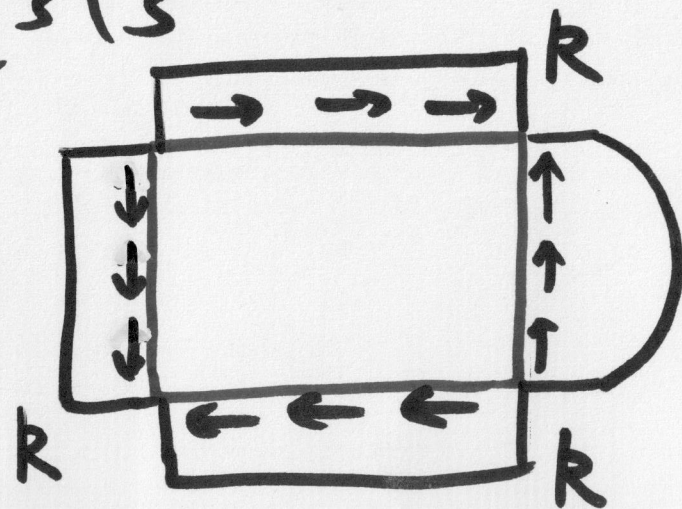
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$(\sigma_{22})_{\text{app}} = 0$
cubic

No load on top/bot face!



$(\sigma_{11})_{\text{app}}$
cubic.



If $b > 0$,
 $k > 0$

P8 Given: For $x_3 > 0$,

$$\sigma_{ij} = \frac{a x_i x_j}{r^5} x_3, \quad r \neq 0, a > 0$$

$$r^2 = x_1^2 + x_2^2 + x_3^2$$

Find: Total force on surface of hemisphere $r = a$.

Method-1: Work in terms of \underline{t} .

$$\phi = x_1^2 + x_2^2 + x_3^2 - a^2 = 0 \text{ defines hemisphere}$$
$$\underline{n} = \underline{\nabla} \phi / |\underline{\nabla} \phi|, \text{ unit normal to hemisphere } r = a$$



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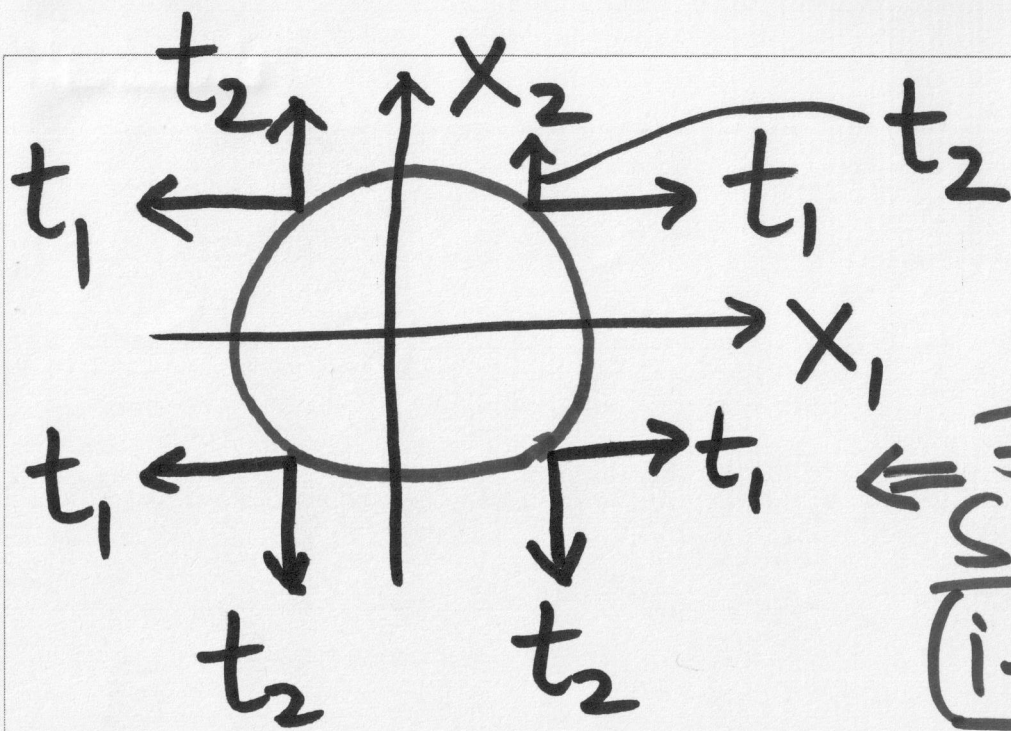
$$\underline{n} = 2x_1 \underline{e}_1 + 2x_2 \underline{e}_2 + 2x_3 \underline{e}_3$$

$$2\sqrt{x_1^2 + x_2^2 + x_3^2} \rightarrow a$$

$$= \frac{x_1}{a} \underline{e}_1 + \frac{x_2}{a} \underline{e}_2 + \frac{x_3}{a} \underline{e}_3$$

$$\underline{t} = \underline{U} \cdot \underline{n} = \frac{1}{a^5} \begin{bmatrix} x_1^2 x_3 & x_1 x_2 x_3 & x_1 x_3^2 \\ x_1 x_2 x_3 & x_2^2 x_3 & x_2 x_3^2 \\ x_1 x_3^2 & x_2 x_3^2 & x_3^3 \end{bmatrix} \begin{Bmatrix} x_1 \\ x_2 \\ x_3 \end{Bmatrix}$$

$$= \frac{1}{a^3} \left\{ \underbrace{x_1 x_3}_{t_1 \text{ odd in } x_1}, \underbrace{x_2 x_3}_{t_2 \text{ odd in } x_2}, x_3^2 \right\}^T$$



Plan of
frustrum of
Sphere
(ie latitude ϕ, z, const)



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For each annular strip of area $2\pi a \sin\phi a d\phi$, lying at latitude ϕ , contributions of t_1, t_2 cancel,
i.e. $F_x = \iint t_1 dA = 0$, $F_y = \iint t_2 dA = 0$

$$F_z = \iint t_3 dA = \frac{1}{a^3} \iint z^2 dA$$

$$z = a \cos \phi, \quad dA = a \sin \phi d\theta.$$

$$F_z = \int_0^{\pi/2} \int_0^{2\pi} \left(\frac{1}{a^3} a^2 \cos^2 \phi \cdot a^2 \sin \phi d\theta \right) d\phi$$

$$= 2\pi a \int_0^{\pi/2} \cos^2 \phi \sin \phi d\phi = \frac{2\pi a}{3}$$

Method-2: $(\underline{\underline{\sigma}})_{R, \theta, \phi} = \underline{\underline{a}} (\underline{\underline{\sigma}})_{x_1, x_2, x_3} \underline{\underline{a}}^T$

Use a, p.2.

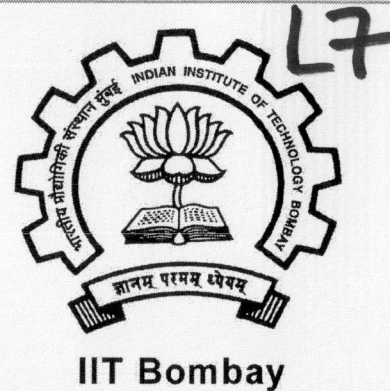
$$\begin{matrix} x'_1 & x'_2 & x'_3 \\ \swarrow & \downarrow & \searrow \\ R & \theta & \phi \end{matrix}$$



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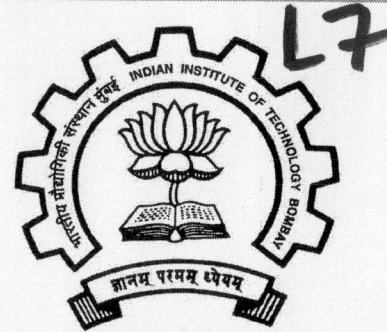
To find forces on surface with \underline{e}_R as normal, need only comp's $\sigma_{RR}, \sigma_{R\theta}, \sigma_{R\phi}$



Transf gives (much algebra),

$$\sigma_{RR} = \frac{\cos\phi}{a}, \quad \sigma_{R\phi} = \sigma_{R\theta} = 0$$

$\therefore \sigma_{RR}$ depends on ϕ only, when integrating $\sigma_{RR} \underline{e}_R dA$, horizontal contributions cancel on opp sides of a latitudinal band $\Rightarrow F_x = F_y = 0$



L7

$$F_z = \int_0^{\pi/2} \int_0^{2\pi} (\sigma_{RR} \underbrace{a d\phi a \sin\phi d\theta}_{dA}) \cos\phi$$

$$= \frac{2\pi a}{3}$$

In general,

$$F_z = \iint_A (\sigma_{RR} dA \cos\phi + \tau_{R\phi} dA \sin\phi)$$

$$F_x = \iint_A (\sigma_{RR} dA s\phi c\theta - \tau_{R\phi} dA c\phi c\theta - \tau_{R\theta} dA s\theta)$$

$$F_y = \iint_A (\sigma_{RR} dA s\phi s\theta - \tau_{R\phi} dA c\phi s\theta + \tau_{R\theta} dA c\theta)$$