

- Marks: **Q1=15, Q2=15, Q3=20.**
- Show all working.
- Show finally only one attempt per question (i.e., cancel out multiple attempts that you do not want to be graded).
- Open notes exam

1. The linear state of strain at a point in a solid is given as

$$e_{ij} = \begin{pmatrix} 1 & -3 & \sqrt{2} \\ -3 & 1 & -\sqrt{2} \\ \sqrt{2} & -\sqrt{2} & 4 \end{pmatrix}$$

Determine:

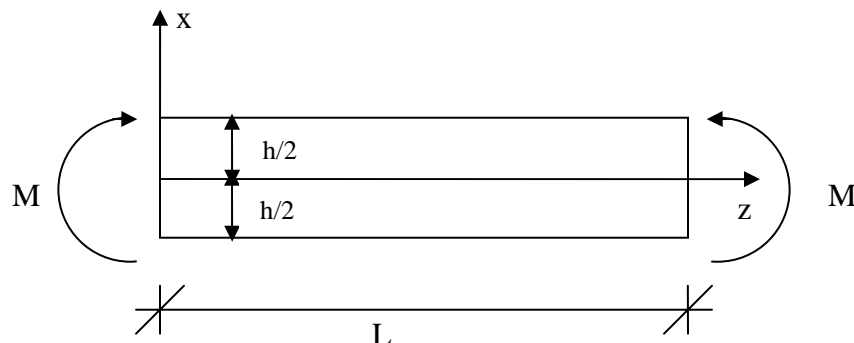
- (a) The extensional strain in the direction $(1, -1, \sqrt{2})$
 (b) The angle after deformation between two line elements originally along $(1, -1, \sqrt{2})$ and $(-1, 1, \sqrt{2})$.

2. Find the direction of the total force acting on the surface of a cylinder due to the stress distribution given below. The cylinder lies in the region $x > 0$, has radius $R = 1$ and length $L = 1$, and its longitudinal axis is the x -axis. The stress distribution is

$$\sigma_{ij} = \begin{pmatrix} 3xy & 5y^2 & 0 \\ 5y^2 & 0 & 2z \\ 0 & 2z & 0 \end{pmatrix}$$

3. Consider a beam of uniform cross-section subject to couple M at its ends, as shown. The couple is applied by means of a linear distribution of σ_{zz} along the boundaries $z = 0$ and $z = L$. Obtain the stresses, strains, and displacements. The beam is restrained from rigid body displacement.

(Hint: From basic solid mechanics, assume a solution of the stress tensor σ_{ij} having only one non-zero component, with the non-zero component varying linearly with the x coordinate only. Check that your stress distribution satisfies boundary condition, equilibrium, compatibility. Then, use this distribution to obtain strains and hence displacements.)



P.1. (a) Linear theory $\rightarrow \epsilon_E = M = e_{ij} n_i n_j = \underline{n}^T \underline{e} \underline{n}$

$$\epsilon_E = \frac{1}{\sqrt{4}} (1 \quad -1 \quad \sqrt{2}) \begin{pmatrix} 6 \\ -6 \\ 6\sqrt{2} \end{pmatrix} = \frac{24}{(\sqrt{4})^2} = 6$$

(If \underline{e} was given as * by 10^{-6} , say, then $\epsilon_E = 6 \times 10^{-6}$).

(b) For direction $\frac{(-1, 1, \sqrt{2})}{\sqrt{4}} = \underline{n}(2)$

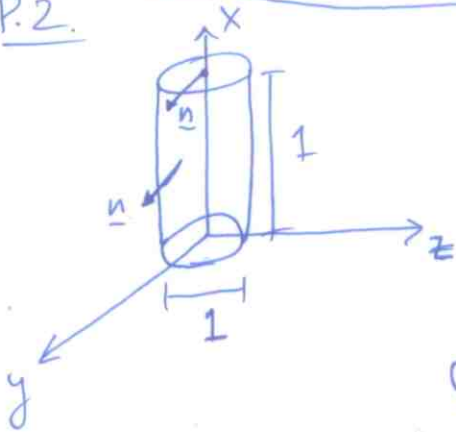
$$\underline{e} \underline{n}(2) = \begin{pmatrix} 1 & -3 & \sqrt{2} \\ -3 & 1 & -\sqrt{2} \\ \sqrt{2} & -\sqrt{2} & 4 \end{pmatrix} \begin{pmatrix} -1 \\ 1 \\ \sqrt{2} \end{pmatrix} \times \frac{1}{2} = \frac{1}{2} \begin{pmatrix} -2 \\ 2 \\ 2\sqrt{2} \end{pmatrix}$$

$$\text{So } \underline{n}(1)^T \underline{e} \underline{n}(2) = \frac{1}{2} (1 \quad -1 \quad \sqrt{2}) \begin{pmatrix} -2 \\ 2 \\ 2\sqrt{2} \end{pmatrix} \times \frac{1}{2} = \frac{1}{4} (-4 + 4) = 0.$$

$$\text{Also } \cos \theta_0 = \underline{n}(1)^T \underline{n}(2) = \frac{1}{2} (1 \quad -1 \quad \sqrt{2}) \begin{pmatrix} -1 \\ 1 \\ \sqrt{2} \end{pmatrix} \times \frac{1}{2} = 0$$

$$\Rightarrow \cos \theta = \frac{\cos \theta_0 + 2 \underline{n}(1)^T \underline{e} \underline{n}(2)}{[(1 + 2 \underline{n}(1)^T \underline{e} \underline{n}(1))^{1/2} (1 + 2 \underline{n}(2)^T \underline{e} \underline{n}(2))^{1/2}]} = \frac{0 + 0}{0} \Rightarrow \theta = 90^\circ \blacktriangleleft$$

P.2.

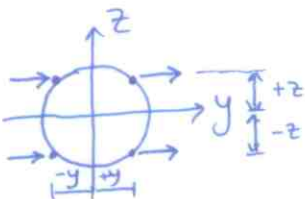


$$\underline{n} = \frac{\nabla \phi}{|\nabla \phi|}, \quad \phi = y^2 + z^2 - 1 = 0.$$

$$\underline{n} = \frac{2(y\mathbf{j} + z\mathbf{k})}{2(y^2 + z^2)^{1/2}} = y\mathbf{j} + z\mathbf{k} = (0, y, z)$$

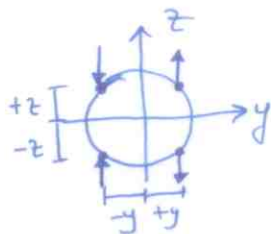
$2(y^2 + z^2) = 1$ on surface.

$$\underline{\sigma} = \underline{\sigma} \underline{n} = \begin{pmatrix} 3xy & 5y^2 & 0 \\ 5y^2 & 0 & 2z \\ 0 & 2z & 0 \end{pmatrix} \begin{pmatrix} 0 \\ y \\ z \end{pmatrix} = \begin{pmatrix} 5y^3 \\ 2z^2 \\ 2yz \end{pmatrix}$$



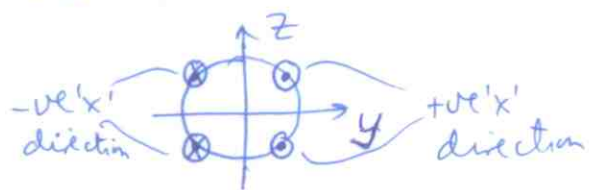
y-component of $\underline{\sigma}$

$$\Rightarrow \sum F_y \neq 0$$



z-component of $\underline{\sigma}$

$$\sum F_z = 0$$



x-component of $\underline{\sigma}$

$$\sum F_x = 0$$

So total force acts in y-direction. \blacktriangleleft

P.3. Assume $\sigma_{zz} = ax$, $a = -\frac{M}{I}$, all other stresses zero. (2)

This solution satisfies equilibrium (\because all other stresses zero, body forces neglected, $\sigma_{zz,z} = 0$), compatibility (\because stresses are linear functions of coordinates, no body forces, so BM compatibility eqns satisfied), and boundary conditions

$$\left\{ \begin{array}{l} \sigma_{zz}|_{x=0,L} = az, \text{ giving rise to } \int_{-h/2}^{h/2} \sigma_{zz} x dx = \frac{ax^3}{3} \Big|_{-h/2}^{h/2} \\ \text{other stresses} = 0 \text{ satisfy } \sigma_{zx}|_{x=0,L} = 0 \\ \sigma_{xx}|_{x=\pm h/2} = 0, \sigma_{zx}|_{x=\pm h/2} = 0. \end{array} \right. = \frac{ah^3}{12} = -\frac{M}{I} \frac{h^3}{12} \text{ for } t=1$$

NOTE -ve sign: BM \downarrow $I = \frac{th^3}{12}$

So THIS IS THE ONLY POSSIBLE SOLUTION.

Constitutive equations (Hooke's law):

$$e_{xx} = e_{yy} = -\frac{\nu}{E} \sigma_{zz} = bx, \quad b = -\frac{\nu}{E} a,$$

$$e_{zz} = \frac{\sigma_{zz}}{E} = cx, \quad c = \frac{a}{E}$$

$$e_{xy} = e_{yz} = e_{zx} = 0.$$

$$u_{x,x} = bx \Rightarrow u_x = \frac{bx^2}{2} + f_x(y,z) \rightarrow \textcircled{1}$$

$$u_{y,y} = bx \Rightarrow u_y = bxy + f_y(x,z) \rightarrow \textcircled{2}$$

$$u_{z,z} = cx \Rightarrow u_z = cxz + f_z(x,y) \rightarrow \textcircled{3}$$

$$e_{xz} = 0 \Rightarrow \frac{\partial f_x}{\partial z} + \frac{\partial f_z}{\partial x} + cz = 0 \rightarrow \textcircled{4}$$

$$e_{yz} = 0 \Rightarrow \frac{\partial f_y}{\partial z} + \frac{\partial f_z}{\partial y} = 0 \rightarrow \textcircled{5}$$

$$e_{xy} = 0 \Rightarrow \frac{\partial f_x}{\partial y} + \frac{\partial f_y}{\partial x} + by = 0 \rightarrow \textcircled{6}$$

$$\frac{\partial \textcircled{4}}{\partial y} + \frac{\partial \textcircled{6}}{\partial z} = 0 \Rightarrow 2 \frac{\partial^2 f_x}{\partial y \partial z} + \frac{\partial^2 f_z}{\partial x \partial y} + \frac{\partial f_y}{\partial x \partial z} = 0$$

$= 0$ from $\frac{\partial \textcircled{5}}{\partial x} = 0.$

$$\Rightarrow f_x = p_y(y) + p_z(z) + R_1 \rightarrow (7)$$

$$(7) \text{ in } (4), (6) \rightarrow p'_z + \frac{df_z}{dx} + Cz = 0 \Rightarrow p'_z + Cz = -\frac{df_z}{dx} = k_2$$

$\underbrace{p'_z + Cz}_{f_n \text{ of } z} \quad \underbrace{-\frac{df_z}{dx}}_{f_n \text{ of } (x,y)}$

$$p'_y + \frac{df_y}{dx} + by = 0 \Rightarrow p'_y + by = -\frac{df_y}{dx} = k_3$$

$\underbrace{p'_y + by}_{f_n \text{ of } y} \quad \underbrace{-\frac{df_y}{dx}}_{f_n \text{ of } (x,z)}$

$$\text{So } p'_z + Cz = k_2 \Rightarrow p_z = k_2 z - \frac{C}{2} z^2 \rightarrow (8)$$

$$p'_y + by = k_3 \Rightarrow p_y = k_3 y - \frac{b}{2} y^2 \rightarrow (9)$$

$$-\frac{df_z}{dx} = k_2 \Rightarrow f_z = -k_2 x + q_y(y) + k_4 \rightarrow (10)$$

→ put in (5)

$$-\frac{df_y}{dx} = k_3 \Rightarrow f_y = -k_3 x + q_z(z) + k_5 \rightarrow (11)$$

$$q'_z + q'_y = 0 \Rightarrow q'_z = -q'_y = k_6$$

$$q_z = k_6 z + k_7 \rightarrow (12)$$

$$q_y = -k_6 y + k_8 \rightarrow (13)$$

$$(1), (7), (8), (9) \rightarrow u_x = b \frac{x^2}{2} - \frac{b}{2} y^2 - \frac{C}{2} z^2 + (k_3 y + k_2 z)$$

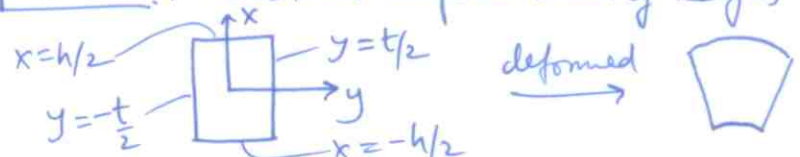
$$(2), (11), (12) \rightarrow u_y = bxy + (k_6 z + k_7 - k_3 x + k_5)$$

$$(3), (10), (13) \rightarrow u_z = cxz - (k_6 y + k_8 - k_2 x + k_4)$$

= 0
 see below

No rigid body translation $\Rightarrow u_x(0,0) = u_y(0,0) = u_z(0,0) = 0$
 $\Rightarrow k_7 + k_5 = k_8 + k_4 = 0$

Remaining linear terms = $(-k_6 \underline{i} + k_2 \underline{j} - k_3 \underline{k}) \times (x \underline{i} + y \underline{j} + z \underline{k})$
 $= \underline{\omega} \times \underline{r} \rightarrow$ set to zero for no rigid body rotation.

Explanation: Consider deformation of edges of rectangular cross section at $z = \text{constant}$.

ie curvature along span and along width in opposite dir. (see POPOV).